Risk and Safety in

Civil, Environmental and Geomatic Engineering

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Contents of Today's Lecture

- Probability theory
- Descriptive statistics
- Uncertainties in engineering decision making
- Probabilistic modelling
- Engineering model building

Overview of Probability Theory

The probability theory provides What are we aiming for ? the basis for the consistent treatment of uncertainties **Data Model estimation** in decision making! **Probabilistic model** Consequences of events **Probabilities of events** We need to be able to **Risks** quantify the probability of events and to update these based on new **Decision Making!** information Swiss Federal Institute of Technology

Interpretation of Probability

States of nature in which we have interest such as:

- a bridge failing due to excessive traffic loads
- a water reservoir being over-filled
- an electricity distribution system "breaking down"
- a project being delayed

are in the following denoted "events".

We are generally interested in quantifying the probability that such events take place within a given "time frame"

Interpretation of Probability

There are in principle three different interpretations of probability

$$P(A) = \lim \frac{N_A}{n_{\rm exp}}$$

for
$$n_{\rm exp} \rightarrow \infty$$

$$P(A) = \frac{n_A}{n_{tot}}$$

$$P(A) =$$
 degree of belief that A will occur

Interpretation of Probability

Consider the probability of getting a "head" when flipping a coin

- Frequentistic
- Classical
- Bayesian

$$P(A) = \frac{510}{1000} = 0.51$$

$$P(A) = \frac{1}{2}$$

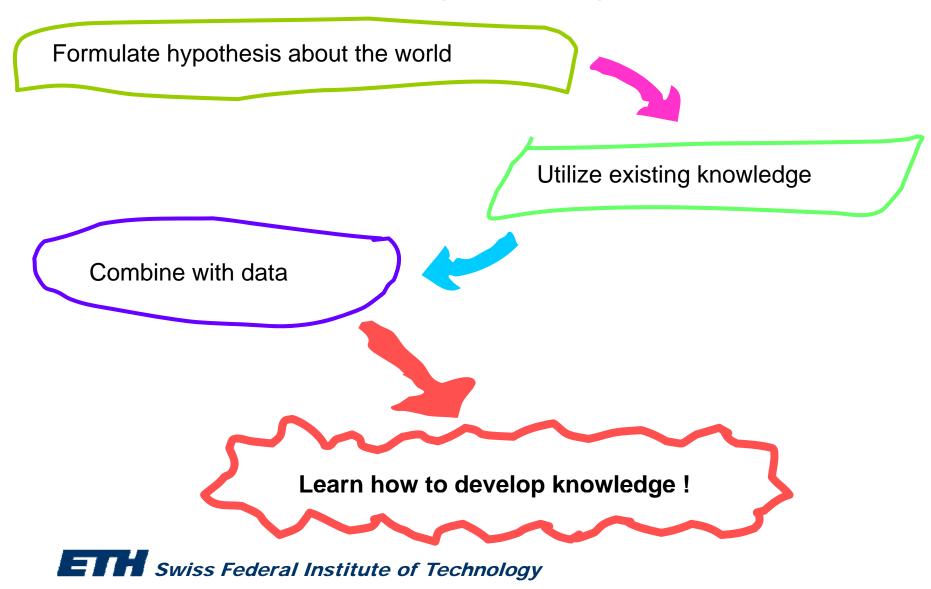
$$P(A) = 0.5$$











Conditional probabilities are of special interest as they provide the basis for utilizing new information in decision making.

The conditional probability of an event E_1 given that event E_2 has occured is written as:

$$P(E_1 | E_2) = \frac{P(E_1 \cap E_2)}{P(E_2)}$$
 Not defined if $P(E_2) = 0$

The events E_1 and E_2 are said to be statistically independent if:

$$P(E_1 | E_2) = P(E_1)$$

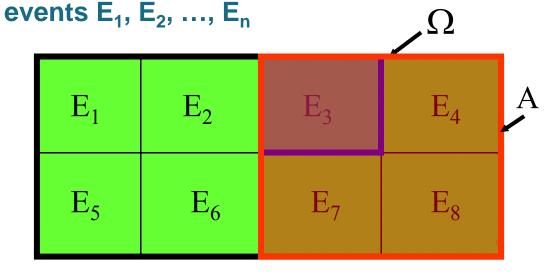
From
$$P(E_1|E_2) = \frac{P(E_1 \cap E_2)}{P(E_2)}$$

it follows that
$$P(E_1 \cap E_2) = P(E_2)P(E_1 \mid E_2)$$

and when E₁ and E₂ are statistically independent it is

$$P(E_1 \cap E_2) = P(E_2)P(E_1)$$

Consider the sample space Ω divided up into n mutually exclusive

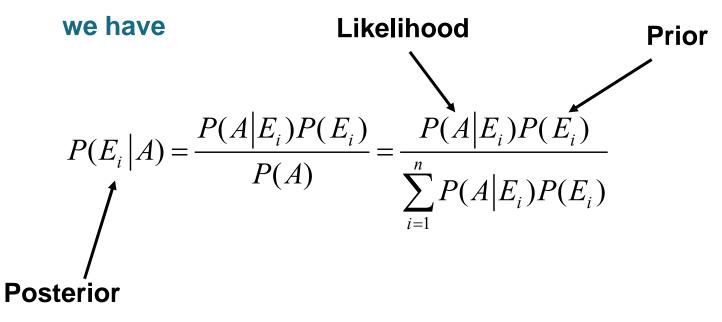


$$P(A) = P(A \cap E_{1}) + P(A \cap E_{2}) + ... + P(A \cap E_{n})$$

$$P(A|E_{1})P(E_{1}) + P(A|E_{2})P(E_{2}) + ... + P(A|E_{n})P(E_{n}) =$$

$$\sum_{i=1}^{n} P(A|E_{i})P(E_{i})$$

as there is $P(A \cap E_i) = P(A|E_i)P(E_i) = P(E_i|A)P(A)$



Bayes Rule



Reverend Thomas Bayes (1702-1764)

Example – inspection of degrading concrete structure

A reinforced concrete structure is considered

It is assumed (known) that the probability that corrosion of the reinforcement has initiated is: P(CI) = 0.01

The state of the reinforcement of the considered beam is unknown and NDE tests are invoked



The quality of the test is specified by the probabilities

- that the test will indicate corrosion given that corrosion has initiated

P(I|CI)

- that the test will indicate corrosion given that corrosion has not initiated

 $P(I|\overline{CI})$

Example – inspection of degrading concrete structure

By comparison of a large number of NDE measurements with the real condition of concrete structures it has been found that

$$P(I|CI) = 0.8$$

$$P(I|\overline{CI}) = 0.1$$

P(CI|I) = ?

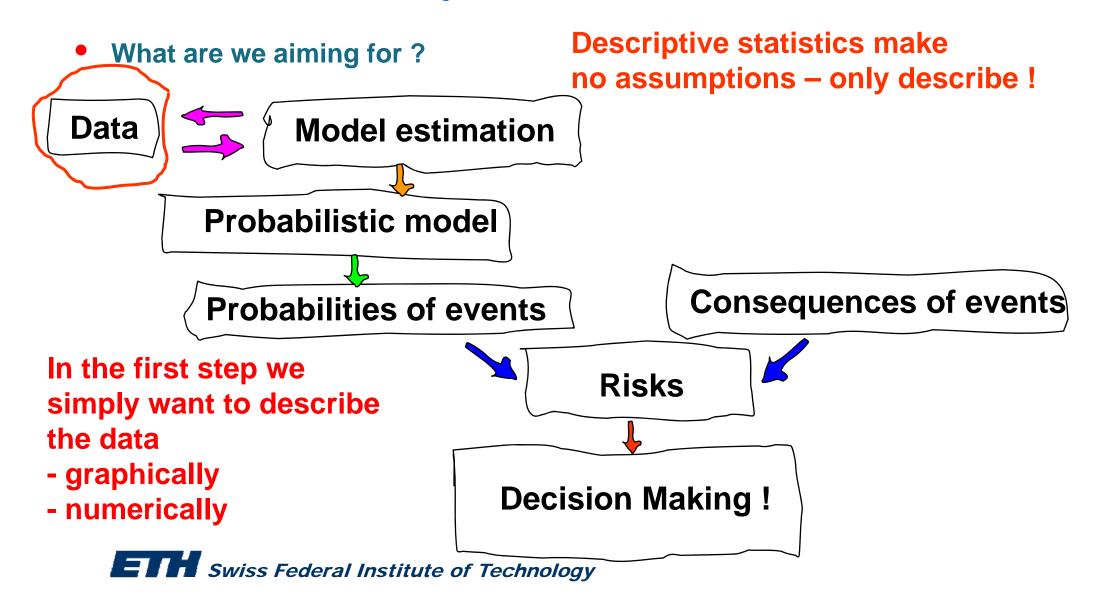
..... No-corrosion

We now seek the probability of corrosion given that we get an indication of corrosion by the NDE inspection i.e.

Posterior
$$P(CI|I) = \frac{P(I|CI)P(CI)}{P(I|CI)P(CI) + P(I|CI)P(CI)}$$

$$P(CI|I) = \frac{0.008}{0.107} = 0.075$$

Overview of Descriptive Statistics



Central measures:

Sample mean:
$$\overline{x} = \frac{1}{n} \sum_{i=1}^{n} x_i$$

If one number should be given to represent a data set typically the sample mean would be chosen

Median: The 0.5 quantile (obtained from ordered data sets, see quantile plots)

Mode: Most frequent value – obtained from histograms

Dispersion measures:

Sample variance:
$$s^2 = \frac{1}{n} \sum_{i=1}^{n} (x_i - \overline{x})^2$$
 S: standard deviation

Indicator of variability around the sample mean

Sample coefficient of variation (CoV): $v = \frac{S}{\overline{x}}$

$$\nu = \frac{s}{\overline{x}}$$

Indicator of variability relative to the sample mean

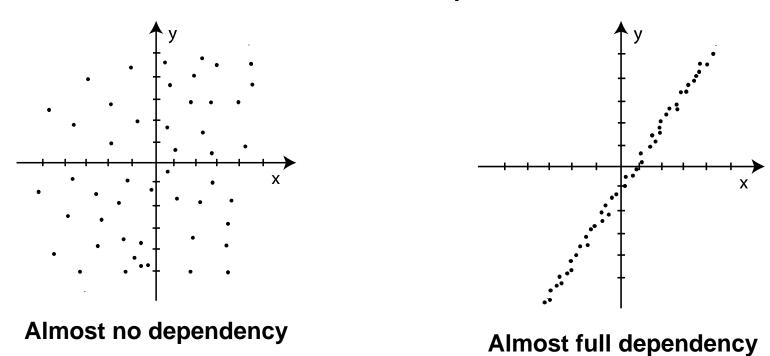
Other measures:

Sample skewness:
$$\eta = \frac{1}{n} \cdot \frac{\sum_{i=1}^{n} (x_i - \overline{x})^3}{s^3}$$
 Measure of symmetry

$$\kappa = \frac{1}{n} \cdot \frac{\sum_{i=1}^{n} (x_i - \overline{x})^4}{s^4}$$
 Measure of peakedness

• Measures of correlation (linear dependency between data pairs):

2-dimensional scatter plots



Measures of correlation (linear dependency between data pairs):

Sample covariance:
$$s^{2}_{XY} = \frac{1}{n} \sum_{i=1}^{n} (x_{i} - \overline{x}) \cdot (y_{i} - \overline{y})$$

The sum will get positive contributions in case of low-low or high-high data pairs

Sample coefficient of correlation:
$$r_{XY} = \frac{1}{n} \frac{\sum_{i=1}^{n} (x_i - \overline{x}) \cdot (y_i - \overline{y})}{s_X \cdot s_Y}$$

 r_{XY} is limited in the interval -1 to +1

Summary:

Central measures:

- sample mean value: The center of gravity of a data set

- sample median: The mid value of a data set

- sample mode: The most frequent value/range of a data set

Dispersion measures:

- sample variance: The distribution around the sample mean

- sample CoV: The variability relative to the sample mean

Other measures:

- sample skewness: The skewness relative to the sample mean

- sample kurtosis: The peakedness around the sample mean

Measures of correlation:

- sample covariance: Tendency for high-high, low-low and high-low

pairs in two data sets

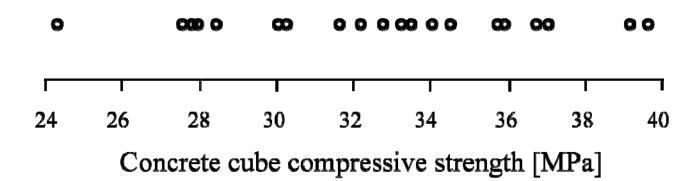
- sample coefficient of correlation :

Normalized coefficient between -1 and +1



 Assume that we have a set of data (observations of concrete compressive strength)

The simplest representation of the data is the one-dimensional scatter plot



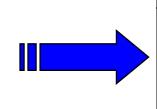
	Unordered	Ordered
i	$\mathcal{X}_{_{i}}$	$oldsymbol{\mathcal{X}}_i^O$
1	35.8	24.4
2	39.2	27.6
3	34.6	27.8
4	27.6	27.9
5	37.1	28.5
6	33.3	30.1
7	32.8	30.3
8	34.1	31.7
9	27.9	32.2
10	24.4	32.8
11	27.8	33.3
12	33.5	33.5
13	35.9	34.1
14	39.7	34.6
15	28.5	35.8
16	30.3	35.9
17	31.7	36.8
18	32.2	37.1
19	36.8	39.2
20	30.1	39.7



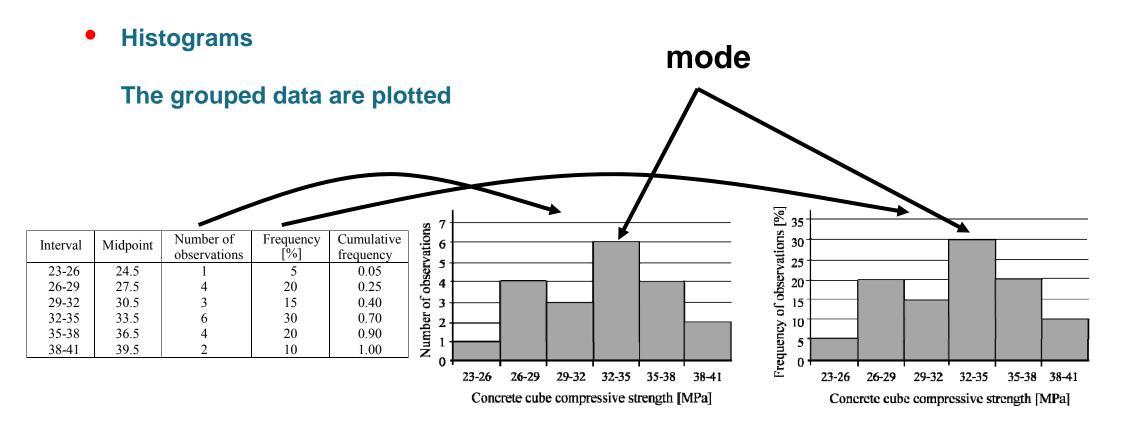
Histograms

The data are grouped into intervals

	Unordered	Ordered
i	$\mathcal{X}_{_{i}}$	$oldsymbol{\mathcal{X}}_i^O$
1	35.8	24.4
2	39.2	27.6
3	34.6	27.8
4	27.6	27.9
5	37.1	28.5
6	33.3	30.1
7	32.8	30.3
8	34.1	31.7
9	27.9	32.2
10	24.4	32.8
11	27.8	33.3
12	33.5	33.5
13	35.9	34.1
14	39.7	34.6
15	28.5	35.8
16	30.3	35.9
17	31.7	36.8
18	32.2	37.1
19	36.8	39.2
20	30.1	39.7



Interval	Midpoint	Number of	Frequency	Cumulative
mervar	wiiapoiiit	observations	[%]	frequency
23-26	24.5	1	5	0.05
26-29	27.5	4	20	0.25
29-32	30.5	3	15	0.40
32-35	33.5	6	30	0.70
35-38	36.5	4	20	0.90
38-41	39.5	2	10	1.00



Simple histogram

Frequency distribution

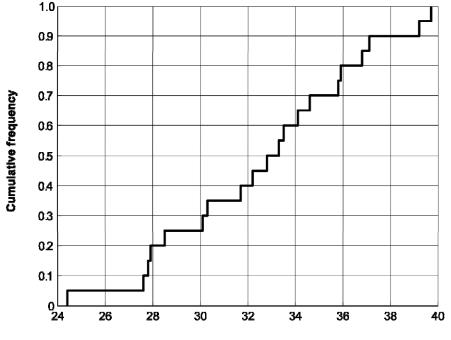


Histograms

The grouped data are plotted



Interval	Midpoint	Number of	Frequency	Cumulative	l
micrvar	Wildpollit	observations	[%]	frequency	l
23-26	24.5	1	5	0.05	Ì
26-29	27.5	4	20	0.25	l
29-32	30.5	3	15	0.40	l
32-35	33.5	6	30	0.70	l
35-38	36.5	4	20	0.90	l
38-41	39.5	2	10	1.00	l



Concrete cube compressive strength (MPa)

Quantile plots

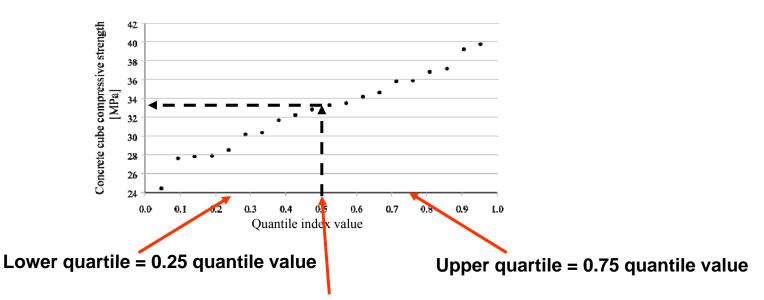
A quantile is related to a percentage.

E.g.: the 0.65 quantile of a given data set of observations is the observation for which 65% of all observations in the data set have smaller values.

Quantile plots are generated by plotting the ordered data against the respective quantile index values.

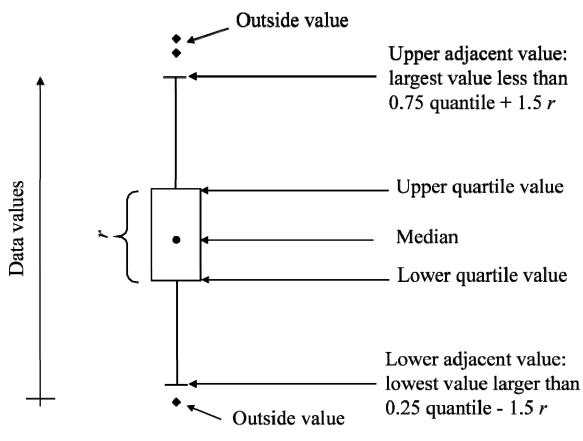
Quantile plots

The quantile index is calculated from the ordered data set as: $v = \frac{i}{1+i\pi}$



	Ordered	
i	$oldsymbol{\mathcal{X}}_i^O$	V
1	24.4	0.048
2	27.6	0.095
3	27.8	0.143
4	27.9	0.190
5	28.5	0.238
6	30.1	0.286
7	30.3	0.333
8	31.7	0.381
9	32.2	0.429
10	32.8	0.476
11	33.3	0.524
12	33.5	0.571
13	34.1	0.619
14	34.6	0.667
15	35.8	0.714
16	35.9	0.762
17	36.8	0.810
18	37.1	0.857
19	39.2	0.905
20	39.7	0.952

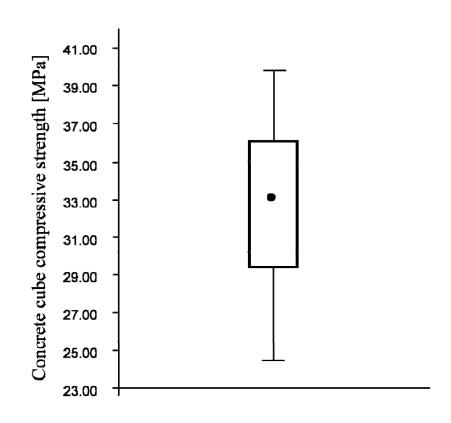
Tukey Box plots



r: Inter-quartile range (50% of data)

Tukey Box plots

Statistic	Value
Lower quartile	29.30
Lower adjacent value	24.40
Median	33.05
Upper adjacent value	39.70
Upper quartile	35.85



Summary

One-dimensional scatter plots

Illustrate the range and distribution of a dataset along one axis; indicate symmetry.

Histograms

Illustrate how the data are distributed over a certain range indicate mode and symmetry.

Quantile plots

Illustrate median, distribution and symmetry.

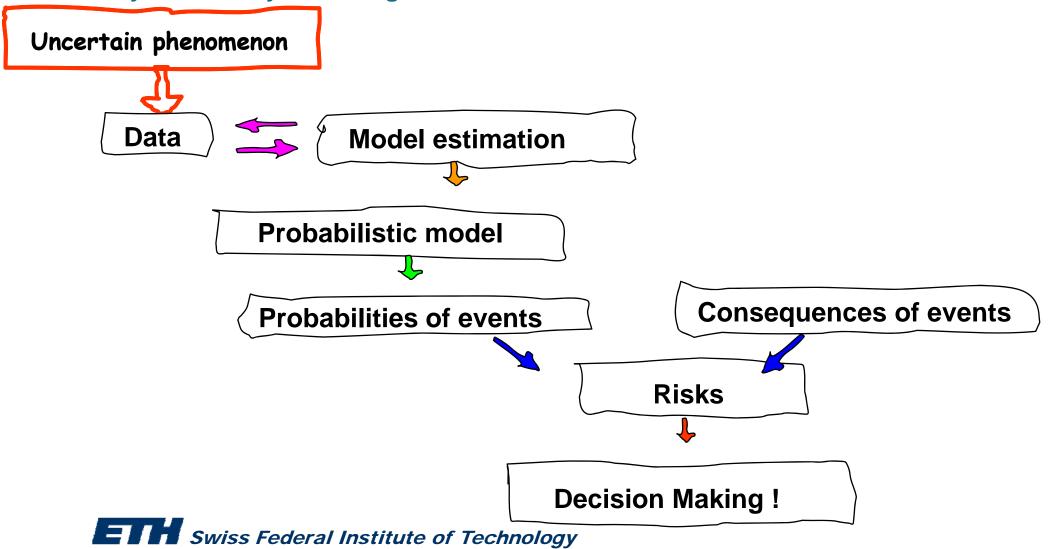
Tukey - Box plots

Illustrate median, upper/lower quartiles, symmetry and distribution.



Overview of Uncertainty Modelling

Why uncertainty modelling



Uncertainties in Engineering Problems

Different types of uncertainties influence decision making

- Inherent natural variability aleatory uncertainty
 - result of throwing dices
 - variations in material properties
 - variations of wind loads
 - variations in rain fall
- Model uncertainty epistemic uncertainty
 - lack of knowledge (future developments)
 - inadequate/imprecise models (simplistic physical modelling)
- Statistical uncertainties epistemic uncertainty
 - sparse information/small number of data

Uncertainties in Engineering Problems

- Consider as an example a dike structure
 - the design (height) of the dike will be determining the frequency of floods
 - if exact models are available for the prediction of future water levels and our knowledge about the input parameters is perfect then we can calculate the frequency of floods (per year) - a deterministic world!
 - even if the world would be deterministic we would not have perfect information about it – so we might as well consider the world as random

Uncertainties in Engineering Problems

In principle the so-called

inherent physical uncertainty (aleatory – Type I)

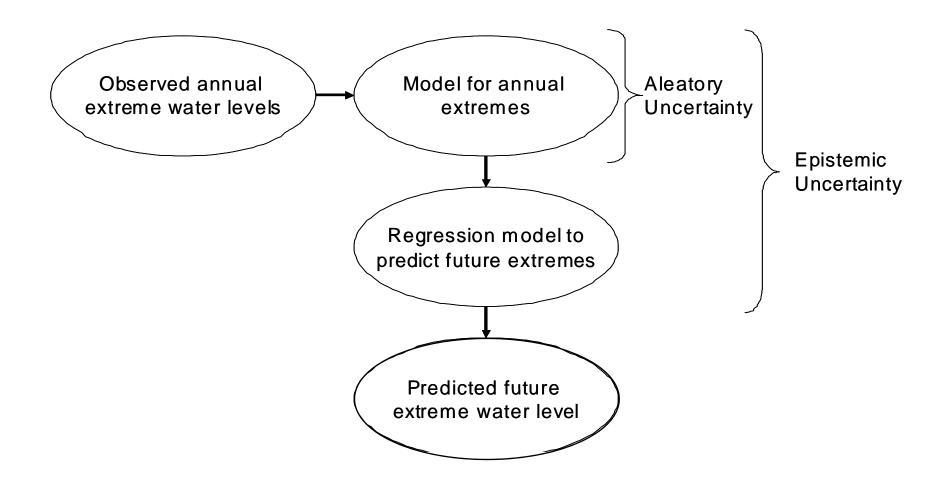
is the uncertainty caused by the fact that the world is random, however, another pragmatic viewpoint is to define this type of uncertainty as

any uncertainty which cannot be reduced by means of collection of additional information

the uncertainty which can be reduced is then the

model and statistical uncertainties (epistemic – Type II)

Uncertainties in Engineering Problems



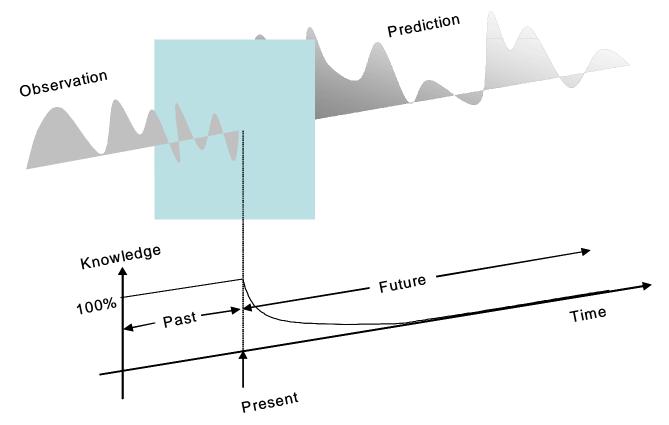
Uncertainties in Engineering Problems

The relative contribution of aleatory and epistemic uncertainty to the prediction of future water levels is thus influenced directly by the applied models

refining a model might reduce the epistemic uncertainty – but in general also changes the contribution of aleatory uncertainty

the uncertainty structure of a problem can thus be said to be scale dependent!

Uncertainties in Engineering Problems



The uncertainty structure changes also as function of time – is thus time dependent!

Probability distribution and density functions

A random variable is denoted with capital letters : X

A realization of a random variable is denoted with small letters : x

We distinguish between

- continuous random variables : can take any value in a given range

- discrete random variables : can take only discrete values



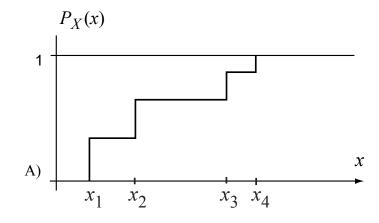
Probability distribution and density functions

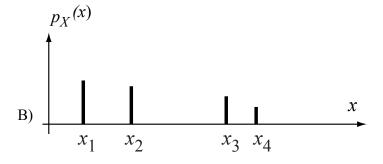
The probability that the outcome of a discrete random variable X is smaller than x is denoted the *probability distribution function*

$$P_X(x) = \sum_{x_i < x} p_X(x_i)$$

The *probability density function* for a discrete random variable is defined by

$$p_X(x_i) = P(X = x)$$





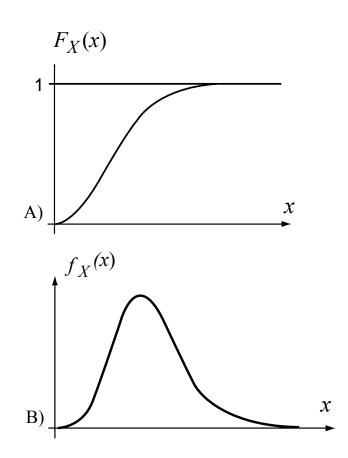
Probability distribution and density functions

The probability that the outcome of a continuous random variable X is smaller than x is denoted the probability distribution function

$$F_X(x) = P(X < x)$$

The probability density function for a continuous random variable is defined by

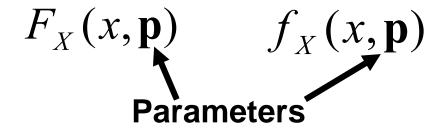
$$f_X(x) = \frac{\partial F_X(x)}{\partial x}$$



Moments of random variables and the expectation operator

Probability distribution and density function can be described in terms of their parameters **p** or their moments

Often we write



The parameters can be related to the moments and visa versa

Moments of random variables and the expectation operator

The ith moment m_i for a continuous random variable X is defined through

$$m_i = \int_{-\infty}^{\infty} x^i \cdot f_X(x) dx$$

The *expected value E[X]* of a continuous random variable *X* is defined accordingly as the first moment

$$\mu_X = E[X] = \int_{-\infty}^{\infty} x \cdot f_X(x) dx$$

Moments of random variables and the expectation operator

The ith moment m_i for a discrete random variable X is defined through

$$m_i = \sum_{j=1}^n x_j^i \cdot p_X(x_j)$$

The expected value E[X] of a discrete random variable X is defined accordingly as the first moment

$$\mu_X = E[X] = \sum_{j=1}^n x_j \cdot p_X(x_j)$$

Moments of random variables and the expectation operator

The standard deviation σ_X of a continuous random variable is defined as the second central moment i.e. for a continuous random variable X we have

$$\sigma_X^2 = \text{Var}[X] = E[(X - \mu_X)^2] = \int_{-\infty}^{\infty} (x - \mu_X)^2 \cdot f_X(x) dx$$
Variance

Mean value

for a discrete random variable we have correspondingly

$$\sigma_X^2 = Var[X] = \sum_{j=1}^n (x_j - \mu_X)^2 \cdot p_X(x_j)$$

Moments of random variables and the expectation operator

The ratio between the standard deviation and the expected value of a random variable is called the *Coefficient of Variation CoV* and is defined as

$$CoV[X] = \frac{\sigma_X}{\mu_X}$$
Dimensionless

a useful characteristic to indicate the variability of the random variable around its expected value

 Typical probability distribution functions in engineering

Normal: Sum of random effects

Log-Normal: Product of random effects

Exponential: Waiting times

Gamma: Sum of waiting times

Beta: Flexible modeling function

Distribution type	Parameters	Moments
Rectangular		
$a \le x \le b$		$\mu = \frac{a+b}{2}$
f(x) = 1	h	2
$f_X(x) = \frac{1}{b-a}$		$\sigma = \frac{b-a}{\sqrt{12}}$
x-a		√12
$F_X(x) = \frac{1}{b-a}$		
$F_X(x) = \frac{x - a}{b - a}$ Normal		
$f_X(x) = \frac{1}{\sigma\sqrt{2\pi}} exp\left(-\frac{1}{2} \left(\frac{x-\mu}{\sigma}\right)^2\right)$	$\mu \\ \sigma > 0$	$\mu \atop \sigma$
$F_X(x) = \frac{1}{\sigma\sqrt{2\pi}} \int_{-\infty}^{x} exp\left(-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^2\right) dx$		
Shifted Lognormal		(
x > ε	λ	$\mu = \varepsilon + \exp\left(\lambda + \frac{\zeta^2}{2}\right)$
$f_X(x) = \frac{1}{(x - \varepsilon)\zeta\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{\ln(x - \varepsilon) - \lambda}{\zeta}\right)^2\right)$	$\zeta > 0$	$\sigma = \exp\left(\lambda + \frac{\zeta^2}{2}\right) \sqrt{\exp(\zeta^2) - 1}$
$F_X(x) = \Phi\left(\frac{\ln(x-\varepsilon) - \lambda}{\zeta}\right)^2$		
Shifted Exponential		1
$x \ge \varepsilon$	ε $\lambda > 0$	$\mu = \varepsilon + \frac{1}{\lambda}$
$f_X(x) = \lambda \exp(-\lambda(x - \varepsilon))$	$\lambda > 0$	1
$F_X(x) = 1 - e^{-\lambda(x-e)}$		$\mu = \varepsilon + \frac{1}{\lambda}$ $\sigma = \frac{1}{\lambda}$
Gamma		p
$x \ge 0$	p > 0 b > 0	$\mu = \frac{1}{b}$
$f_X(x) = \frac{b^p}{\Gamma(n)} \exp(-bx)x^{p-1}$	b > 0	\sqrt{n}
$\int_{X} (x) - \frac{1}{\Gamma(p)} \exp(-\nu x) x^{2}$		$\mu = \frac{p}{b}$ $\sigma = \frac{\sqrt{p}}{b}$
$\Gamma(bx,p)$		D
$F_X(x) = \frac{\Gamma(bx, p)}{\Gamma(p)}$		
Beta		ľ
$a \le x \le b, r, t \ge 1$	а	$\mu = a + (b - a) \frac{r}{r + 1}$
$\Gamma(r+t) (x-a)^{r-1} (b-x)^{t-1}$	1 h	
$f_X(x) = \frac{\Gamma(r+t)}{\Gamma(r) \cdot \Gamma(t)} \frac{(x-a)^{r-1} (b-x)^{t-1}}{(b-a)^{r+t-1}}$	r > 1 $t > 1$	$\sigma = \frac{b-a}{r+t} \sqrt{\frac{rt}{r+t+1}}$
$F_X(x) = \frac{\Gamma(r+t)}{\Gamma(r) \cdot \Gamma(t)} \cdot \int_a^u \frac{(u-a)^{r-1} (b-u)^{t-1}}{(b-a)^{r+t-1}} du$		

The Normal distribution

The analytical form of the Normal distribution may be derived by repeated use of the result regarding the probability density function for the sum of two random variables

The normal distribution is very frequently applied in engineering modelling when a random quantity can be assumed to be composed as a sum of a number of individual contributions.

A linear combination S of n Normal distributed random variables X_i , i = 1, 2, ..., n is thus also a Normal distributed random variable

$$S = a_0 + \sum_{i=1}^n a_i X_i$$

• The Normal distribution:

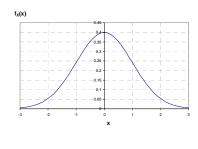
In the case where the mean value is equal to zero and the standard deviation is equal to 1 the random variable is said to be *standardized*.

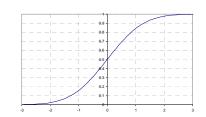
$$Z = \frac{X - \mu_X}{\sigma_X}$$
 Standardized random variable

$$f_{Z}(z) = \varphi(z) = \frac{1}{\sqrt{2\pi}} \exp\left(-\frac{1}{2}z^{2}\right)$$

$$F_{Z}(z) = \Phi(z) = \frac{1}{\sqrt{2\pi}} \int_{-\pi}^{z} \exp\left(-\frac{1}{2}x^{2}\right) dx$$

Standard normal





- Random quantities may be "time variant" in the sense that they take new values at different times or at new trials.
 - If the new realizations occur at discrete times and have discrete values the random quantity is called a random sequence

failure events, traffic congestions,...

- If the new realizations occur continuously in time and take continuous values the random quantity is called a random process or stochastic process

wind velocity, wave heights,...

Random sequences

The Poisson counting process is one of the most commonly applied families of probability distributions applied in reliability theory

The process N(t) denoting the number of events in a time interval [0;t[is called a Poisson process if the following conditions are fulfilled:

- 1) the probability of one event in the interval $[t,t+\Delta t[$ is asymptotically proportional to Δt .
- 2) the probability of more than one event in the interval $[t, t + \Delta t]$ is a function of higher order of Δt for $\Delta t \rightarrow 0$.
- 3) events in disjoint intervals are mutually independent.

Random sequences

The probability distribution function of the (waiting) time till the first event T_1 is now easily derived recognizing that the probability of $T_1 > t$ is equal to $P_0(t)$ we get:

$$F_{T_{I}}(t_{I})=1-P_{0}(t_{I})$$

$$=1-exp(-\int_{0}^{t}v(\tau)d\tau)$$

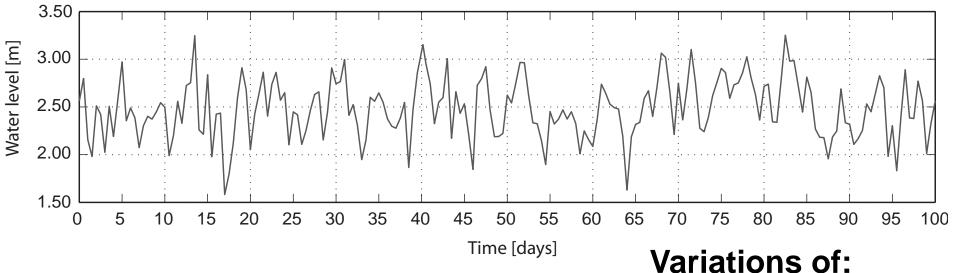


Exponential probability distribution Exponential probability density

$$\oint_{T_1} (t_1) = v \cdot exp(-vt)$$

Continuous random processes

A continuous random process is a random process which has realizations continuously over time and for which the realizations belong to a continuous sample space.



water levels wind speed rain fall

Continuous random processes

The mean value of the possible realizations of a random process is given as:

$$\mu_X(t) = E[X(t)] = \int_{-\infty}^{\infty} x \cdot f_X(x,t) dx$$
Function of time!

The correlation between realizations at any two points in time is given as:

$$R_{XX}(t_1, t_2) = E[X(t_1)X(t_2)] = \int_{-\infty-\infty}^{\infty} \int_{-\infty-\infty}^{\infty} x_1 \cdot x_2 \cdot f_{XX}(x_1, x_2; t_1, t_2) dx_1 dx_2$$

Auto-correlation function – refers to a scalar valued random process

Extreme Value Distributions

In engineering we are often interested in extreme values i.e. the smallest or the largest value of a certain quantity within a certain time interval e.g.:

The largest earthquake in 1 year

The highest wave in a winter season

The largest rainfall in 100 years

Extreme Value Distributions

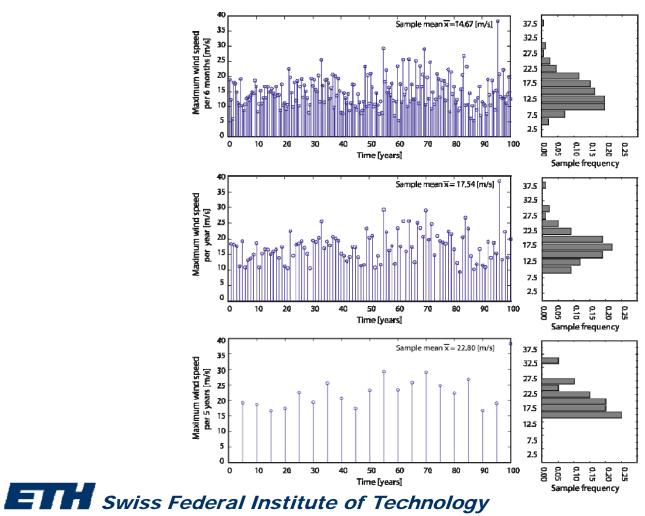
We could also be interested in the smallest or the largest value of a certain quantity within a certain volume or area unit e.g.:

The largest concentration of pesticides in a volume of soil

The weakest link in a chain

The smallest thickness of concrete cover

Extremes of a random process:



Return period for extreme events:

The return period for extreme events T_R may be defined as

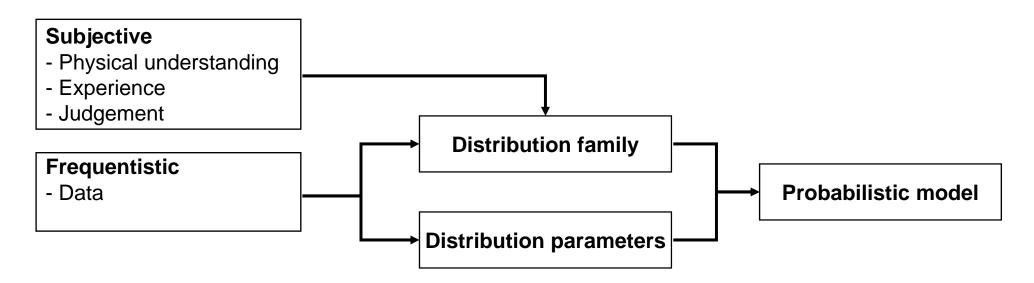
$$T_R = n \cdot T = \frac{1}{(1 - F_{X,T}^{\text{max}}(x))} T$$

If the probability of exceeding x during a reference period of 1 year is 0.01 then the return period for exceedances is

$$T_R = n \cdot T = \frac{1}{0.01} \cdot 1 = 100 \cdot 1 = 100$$

Different types of information is used when developing engineering models

- subjective information
- frequentististic information

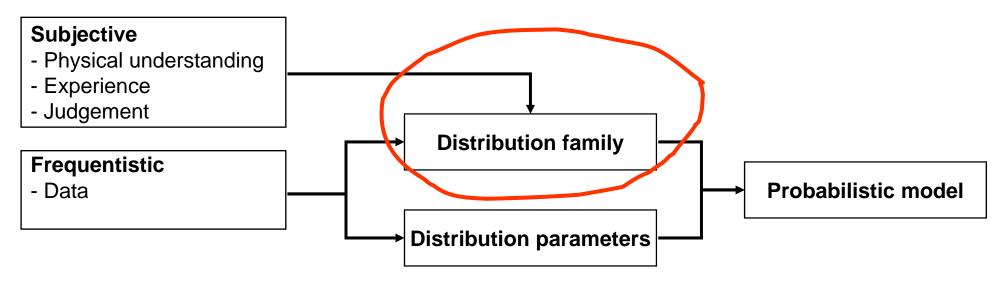


Model building may be seen to consist of five steps

- 1) Assessment and statistical quantification of the available data
- 2) Selection of distribution function
- 3) Estimation of distribution parameters
- 4) Model verification
- 5) Model updating

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Estimation and Model Building

Selection of probability distribution function

In engineering application it is often the case that

the available data is too sparse

to be able to support/reject the hypothesis of a given probability distribution – with a reasonable significance

Therefore it is necessary to use common sence i.e.:

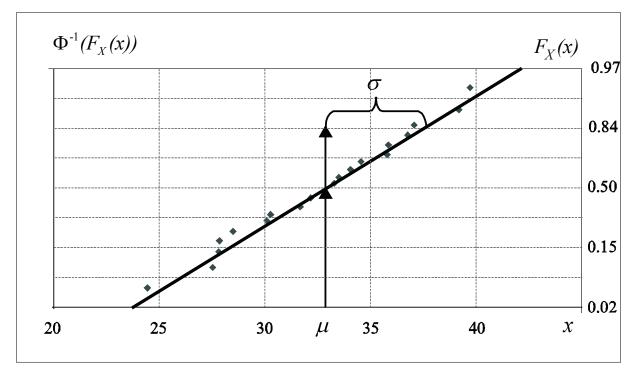
First to consider physical reasons for selecting a given distribution

Thereafter to check if the available data are in gross contradiction with the selected distribution

Estimation and Model Building

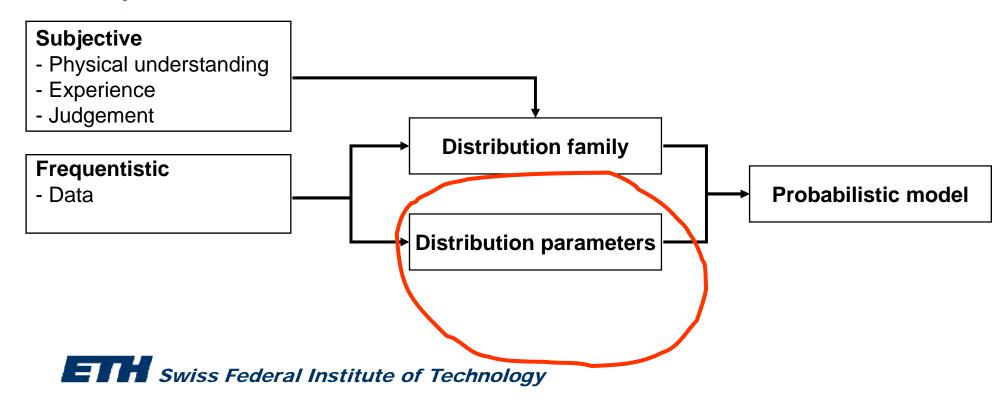
Model selection by use of probability paper

Plotting the sample probability distribution function in the probability paper yields



Different types of information is used when developing engineering models

- subjective information
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We assume that we have identified a plausible family of probability distribution functions – as an example :

Normal Distribution

Weibull distribution

$$f_X(x) = \frac{1}{\sigma\sqrt{2\pi}} exp\left(-\frac{1}{2}\left(\frac{x+\mu}{\sigma}\right)^2\right) \qquad f_X(x) = \frac{k}{u-\varepsilon}\left(\frac{x+\varepsilon}{u-\varepsilon}\right)^{k-1} \exp\left(-\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^k\right)$$

and thus now need to determine/estimate its parameters

$$\mathbf{\theta} = (\theta_1, \theta_2, ..., \theta_k)^T$$

The method of moments (MoM)

To start with we assume that we have data on the basis of which we can estimate the distribution parameters $\hat{\mathbf{x}} = (\hat{x}_1, \hat{x}_2, ..., \hat{x}_n)^T$

The idea behind the method of moments is to determine the distribution parameters such that the sample moments (from the data) and the analytical moments (from the assumed distribution) are identical.

$$m_j = \frac{1}{n} \sum_{i=1}^n x_i^j$$

$$\lambda_{j} = \int_{-\infty}^{\infty} x^{j} \cdot f_{X}(x|\mathbf{\theta}) dx$$
$$= \lambda_{j}(\theta_{1}, \theta_{2}, ..., \theta_{k})$$

Sample moments

Analytical moments



The Maximum Likelihood Method (MLM)

The idea behind the method of maximum likelihood is that

the parameters are determined such that the likelihood of the observations is maximized.

The likelihood can be understood as the probability of occurrence of the observed data conditional on the model.

The Maximum Likelihood Method may seem to be more complicated than the Method of Moments but has a number of attractive properties.

Summary

Method of Moments provides point estimates of the parameters.

- No information about the uncertainty with which the parameter estimates are associated.

Maximum Likelihood Method provides point estimates of the parameters.

- Full distribution information – normal distributed parameters, mean values and covariance matrix.