Basic Statistics and Probability Theory in

Civil, Surveying and Environmental Engineering

Prof. Dr. Michael Havbro Faber Swiss Federal Institute of Technology ETH Zurich, Switzerland



Contents of Today's Lecture

- The organisation of the lecture practical stuff
- Why statistics and probability in engineering?
- Decision Problems in Engineering
- Examples
- The lecture program

What do we offer to you ?

- It is our intention to provide you to the best of our abilities
 - Motivation and overview of context
 - Targeted presentation of required knowledge
 - Guidance on self study
 - Help on training your abilities
 - Help on your self evaluation
- We are here for you and we take this statement seriously

Structure and organization of the course

- 13 weekly lectures of each two sessions of 45 minutes
- 11 weekly exercise tutorials of each two sessions of 45 minutes
- 2 assessments of each 90 minutes
- Self study estimated to 4 times by 45 minutes per week



The course's web page

http://www.ibk.ethz.ch/fa/education/ss_statistics

What can you find there?

- Course's program and timetable
- Tutorial's timetable
- Script (downloadable/printable)
- Exercises/Solutions for the exercise tutorials (downloadable/printable)
- Presentations of the lecture and of the exercise tutorial (uploaded a day before the respective day)
- Videos of the lecture (uploaded the day after the lecture)
- Glossary (German-English terms)
- Links to helpful web pages
- Past examination papers
- Your exercise tutorial class and group!

Organization of the Lecture

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When??
Normally...Tuesdays 8-10
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Where?? HIL E1

Exceptions: Thursday 22.03.07 8-10 HPH G 3 (lecture instead of exercise tutorial) Other exceptions: Check the course's program!

Script (English)
 Download from the course's web page

Organization of the Exercise Tutorials





Organization of the Exercise tutorial

- When??: Normally....Thursday 8-10
- Where?? HPH G 3 HCI H 2.1 HCI D 8 HCI D 2
- Where do I go???
 find out in the "Group lists" link on the course's web page
- Exceptions....©
 First tutorial: Tuesday 27.03.07
 Where???: HIL E1 HIL B 21 HIL D 10.2 HIL F 10.3

Organization of the Exercise Tutorials

2 or more exercises will be presented in steps (based on the content of the latest lecture)

1 or more solution(s) of exercises shown in steps in the last tutorial









Organization of the Exercise Tutorials



What do we expect from you?

- Take advantage of the help we provide to you
 - benefit from the lectures
 - benefit from the exercise tutorials
 - benefit from the help of the assistants and professor (office hours)
- Tips and tricks
 - prepare yourself for the lectures
 - ask questions
 - try to understand the topics rather than prepare for examination
 - be curious, interested, open minded but critical to what we tell you

What do we expect from you?



Mode of assessment

- <u>Two assessments during the semester</u> one midterm (03.05.07) the other one towards the end of the course (14.06.07)
- <u>Final Exam</u> October/March....

Final mark=
$$\frac{1}{3}$$
(two assessments) + $\frac{2}{3}$ (final exam)

Programmable calculators are strictly not allowed! Open book assessments and final exam©

Read carefully all the information in the "Preamble" of the script!! If you have any questions ask!



- What do engineers do ?
 - Plan, design, build, maintain and decommission

Infrastructure

Roads, water supply systems, tunnels, sewage systems, waste deposits, power supply systems, channels

Structures

houses, hospitals, schools, industry buildings, dams, powerplants, wind turbines, offshore platforms

- Safeguard
 - people
 - environment SUSTAINABLE DEVELOPMENT !
 - assets

from natural and man made hazards

What are engineers working with ?



How do engineers work with the real world ?



We model the real world to the "best" of our knowledge

• How do engineers use knowledge

In a perfectly known world



How do engineers establish knowledge





• An example where models were not too representative







All activities are associated with uncertainties

Activities are e.g.

- Transport
- Work
- Sport

but also





- Construction and operation of production and infrastructure projects
- Research and development







Every day we must make decisions in regard to activities associated with uncertainties



Every one of these activities is associated with uncertainties We all have an opinion regarding the associated risks We have gut Feelings !

How far can we get with gut feelings ?



An example

After all - maybe it is not so "straight forward" to comprehend uncertainties ? What can we learn from the past ?

Disasters and accidents have always occurred Some examples





Tacoma Narrows, Washington, 1940

Fort Mayer, Virginia, 1908



Disasters and accidents have always occurred Some examples



Concord, North Carolina, 2000



Concorde, Paris, 2000



Disasters and accidents have always occurred Some examples



Kobe, 1995



Disasters and accidents have always occurred

Some examples



Canada, 1993

Open questions

- did we realise the risks ?
- are the consequences acceptable ?

Risk assessment, within the framework of decision analysis, provides a basis for rational decision making subject to uncertain and / or incomplete information

Thereby we can take into account, in a consistent manner, the prevailing uncertainties and quantify their effect on risks

Thus we may find answers to the following questions

- How large is the risk associated with a given activity ?
- How may we reduce and / or mitigate risks ?
- How much does it cost to reduce and / or mitigate risks ?
- What risks must we accept what can we afford ?

- Risk is a characteristic of an activity relating to all possible events n_E which may follow as a result of the activity
- The risk contribution R_{E_i} from the event E_i is defined through the product between
- the Event probability P_{E_i}

and

the Consequences of the event C_{E_i}

The Risk associated with a given activity R_A may then be written as

$$R_{A} = \sum_{i=1}^{n_{E}} R_{E_{i}} = \sum_{i=1}^{n_{E}} P_{E_{i}} \cdot C_{E_{i}}$$

Decision Problems in Engineering

Uncertainties must be considered in the decision making throughout all phases of the life of an engineering facility



The Frigg Field - built 1972-1978

- TCP2 TP1 CDP1

According to international conventions the structures must be decommissioned

Each structure :

Weight : 250000 t Costs : 200 - 600 Mio. SFr



None of the platforms were designed for decommissioning !

• The decision problem

Decommissioning/removal taking into account

- Safety of personnel
- Safety of the environment
- Costs
- Interest groups

Greenpeace Fishers IMO

IAT

LAT

- Three options are considered
 - "Refloat" and demolition Onshore
 - "Refloat" and demolition Offshore
 - Removal to a free passage of 55 m depth
- The approach
 - Identification of hazard scenarios
 chronologically
 - Quantification of occurrence probabilities
 - Quantification of consequences
 - Applied approach Bayesian Nets





Results of the decision analysis



 How much to invest before a satisfactorily level of probability of mission success has been reached
• Structural Design

Exceptional structures are often associated with structures of "Extreme Dimensions"



Great Belt Bridge under Construction



Concept drawing of the Troll platform

Structural Design

or associated with structures fulfilling "New and Innovative Purposes"









Illustrations of the ARIANE 5 rocket

Concept drawing of Floating Production, Storage and Offloading unit



for rebuilding and strengthening

in regard to possible earthquakes

Inspection and Maintenance Planning ٠

Due to

- operational loading
- environmental exposure

structures will always to some degree be exposed to degradation processes such as

- fatigue
- corrosion
- scour
- wear



Why Statistics and Probability in Engineering?

- In summary
 - statistics and probability theory is needed in engineering to
 - quantify the uncertainty associated with engineering models
 - evaluate the results of experiments
 - assess importance of measurement uncertainties
 - safe guard

safety for persons qualities of environment assets

ENHANCE DECISION MAKING



Organisation of the Lecture

Module A

Engineering decisions under uncertainty



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Rooms information

Before...

Group	Tutorial 1	Tutorials 2-9 and 11	Tutorial 10
Е	HIL D 10.2	HCI D 2	To be announced
Н	HIL B 21	HCI H 2.1	
K	HIL F 10.3	HCI D 8	
V	HIL E 1	HPH G 3	

Now...

Group	Tutorial 1	Tutorials 2-9 and11	Tutorial 10
Е	HCI D 2	HCI D 2	
Н	HPT C103	HCI H 2.1	To be announced
K	HIL F 10.3	HCI D 8	
V	HIL E 1	HPH G 3	

Time starting (Lecture/Tutorials):

HIL: 8 Physics/Chemistry Buildings: 7.45

Contents of Todays Lecture

- Risk and Motivation for Risk Assessment
- Overview of Probability Theory
- Interpretation of Probability
- Sample Space and Events
- The three Axioms of Probability Theory
- Conditional Probability and Bayes's Rule

Why Statistics and Probability in Engineering?

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Greenpeace Fishers IMO

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statistics and probability theory is needed in engineering to

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safety for persons
qualities of environment
assets
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ENHANCE DECISION MAKING



Overview of Probability Theory



• What is Probability ?

We all have some notion of probability !



States of nature of which we have interest such as:

- a bridge failing due to excessive traffic loads
- a water reservoir being over-filled
- an electricity distribution system "falling out"
- a project being delayed

are in the following denoted "events"

we are generally interested in quantifying the probability that such events take place within a given "time frame"



• There are in principle three different interpretations of probability

- Frequentistic
$$P(A) = \lim \frac{N_A}{n_{exp}}$$
 for $n_{exp} \to \infty$ - Classical $P(A) = \frac{n_A}{n_{tot}}$ - Bayesian $P(A) =$ degree of belief that A will occur



Consider the probability of getting a "head" when flipping a coin

- Frequentistic
$$P(A) = \frac{510}{1000} = 0.51$$

- Classical $P(A) = \frac{1}{2}$



- Bayesian P(A) = 0.5



The set of all possible outcomes of the state of nature e.g. concrete compressive strength test results is called the sample space Ω . For concrete compressive strength test results the sample space can be written as $\Omega =]0; \infty[$

A sample space can be continuous or discrete.

Typically we illustrate the sample space and events using Venn diagrams





An event is a sub-set of the sample space

- if the sub-set is empty the event is impossible
- if the sub-set contains all of the sample space the event is certain

Consider the two events E_1 and E_2 : The sub-set of sample points belonging to the event E_1

and/or the event E_2 is called the union of E_1 and E_2 and is written as : $E_1 \cup E_2$



An event is a sub-set of the sample space

- if the sub-set is empty the event is impossible
- if the sub-set contains all of the sample space the event is certain

Consider the two events E_1 and E_2 : The sub-set of sample points belonging to the event E_1 and the event E_2 is called the intersection of E_1 and E_2 and is written as: $E_1 \cap E_2$



The event containing all sample points in Ω not included in the event E is called the complementary event to Eand written as : \overline{E}

It follows that $E \cup \overline{E} = \Omega$

and $E \cap E = \emptyset$





It can be show that the intersection and union operations obey the following commutative, associative and distributive laws:

 $E_{I} \cap E_{2} = E_{2} \cap E_{I}$ Commutative law $E_{I} \cap (E_{2} \cap E_{3}) = (E_{I} \cap E_{2}) \cap E_{3}$ $E_{I} \cup (E_{2} \cup E_{3}) = (E_{I} \cup E_{2}) \cup E_{3}$ Associative law $E_{I} \cap (E_{2} \cup E_{3}) = (E_{I} \cap E_{2}) \cup (E_{I} \cap E_{3})$ $E_{I} \cup (E_{2} \cap E_{3}) = (E_{I} \cup E_{2}) \cap (E_{I} \cup E_{3})$ Distributive law

From the commutative, associative and distributive laws the so-called De Morgan's laws may be derived:

 $E_1 \cap E_2 = E_2 \cap E_1$ $E_1 \cap (E_2 \cap E_3) = (E_1 \cap E_2) \cap E_3$ $E_1 \cap E_2 = \overline{\overline{E}_1 \cup \overline{E}_2}$ $E_1 \cup E_2 = \overline{\overline{E}_1 \cap \overline{E}_2}$ $E_1 \bigcup (E_2 \bigcup E_3) = (E_1 \bigcup E_2) \bigcup E_3$ $E_1 \cap (E_2 \cup E_3) = (E_1 \cap E_2) \cup (E_1 \cap E_3)$ $E_1 \bigcup (E_2 \cap E_3) = (E_1 \bigcup E_2) \cap (E_1 \bigcup E_3)$

The Three Axioms of Probability Theory

The probability theory is built up on – only – three axioms due to Kolmogorov:

Axiom 1: $0 \le P(E) \le 1$

Axiom 2: $P(\Omega) = 1$

Axiom 3:

$$P\left(\bigcup_{i=1}^{n} E_{i}\right) = \sum_{i=1}^{n} P(E_{i})$$

When E_1 , E_2 ,... are mutually exclusive



Conditional Probability and Bayes's Rule





Conditional Probability and Bayes's Rule





Conditional Probability and Bayes's Rule




Conditional Probability and Bayes's Rule



Conditional Probability and Bayes's Rule

Conditional probabilities are of special interest as they provide the basis for utilizing new information in decision making.

The conditional probability of an event E_1 given that event E_2 has occured is written as:

$$P(E_1|E_2) = \frac{P(E_1 \cap E_2)}{P(E_2)}$$
 Not defined if $P(E_2) = 0$

The events E_1 and E_2 are said to be statistically independent if:

$$P(E_1 | E_2) = P(E_1)$$

Conditional Probability and Bayes's Rule From $P(E_1|E_2) = \frac{P(E_1 \cap E_2)}{P(E_2)}$

it follows that $P(E_1 \cap E_2) = P(E_2)P(E_1 | E_2)$

and when E_1 and E_2 are statistically independent there is

 $P(E_1 \cap E_2) = P(E_2)P(E_1)$



Conditional Probability and Bayes's Rule

Consider the sample space Ω divided up into n mutually exclusive events E_1 , E_2 , ..., E_n



$$P(A) = P(A \cap E_1) + P(A \cap E_2) + \dots + P(A \cap E_n)$$

$$P(A|E_1)P(E_1) + P(A|E_2)P(E_2) + \dots + P(A|E_n)P(E_n) =$$

$$\sum_{i=1}^{n} P(A|E_i)P(E_i)$$

Conditional Probability and Bayes's Rule

as there is $P(A \cap E_i) = P(A | E_i) P(E_i) = P(E_i | A) P(A)$





Reverend Thomas Bayes (1702-1764)

Basic Statistics and Probability Theory in

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Contents of Todays Lecture

- Overview of descriptive statistics
- Numerical summaries
 - Central measures
 - Dispersion measures
 - Other measures
 - Measures of correlation
- Graphical representations
 - One-dimensional scatter plots
 - Histograms
 - Quantile plots
 - Tukey Box plots
 - Q-Q plots and Tukey mean-difference plot

Overview of Descriptive Statistics



Central measures:

Sample mean :
$$\overline{x} = \frac{1}{n} \sum_{i=1}^{n} x_i$$

If one number should be given to represent a data set typically the sample mean would be chosen

Median : The 0.5 quantile (obtained from ordered data sets, see quantile plots)

Mode : Most frequent value - obtained from histograms ETH Swiss Federal Institute of Technology

• Dispersion measures:

Sample variance:
$$s^2 = \frac{1}{n} \sum_{i=1}^{n} (x_i - \overline{x})^2$$
 s: standard deviation

Indicator of variability around the sample mean

Sample coefficient of variation (CoV):
$$V = \frac{S}{\overline{x}}$$

Indicator of variability relative to the sample mean

• Other measures:

Sample skewness:

$$\eta = \frac{1}{n} \cdot \frac{\sum_{i=1}^{n} (x_i - \overline{x})^3}{s^3}$$

Sample kurtosis

$$\kappa = \frac{1}{n} \cdot \frac{\sum_{i=1}^{n} (x_i - \overline{x})^4}{s^4}$$

Measure of peakedness



• Measures of correlation (linear dependency between data pairs):

2-dimensional scatter plots



Almost no dependency



Almost full dependency



• Measures of correlation (linear dependency between data pairs):

Sample covariance:

$$s_{XY} = \frac{1}{n} \sum_{i=1}^{n} (x_i - \overline{x}) \cdot (y_i - \overline{y})$$

The sum will get positive contributions in case of low-low or high-high data pairs

Sample coefficient of correlation:
$$r_{XY} = \frac{1}{n} \frac{\sum_{i=1}^{n} (x_i - \overline{x}) \cdot (y_i - \overline{y})}{s_X \cdot s_Y}$$

 r_{XY} is limited in the interval -1 to +1



• Summary:

Central measures:

- sample mean value:
- sample median:
- sample mode:

Dispersion measures:

- sample variance:
- sample CoV:

Other measures:

- sample skewness:
- sample kurtosis:

The center of gravity of a data set The mid value of a data set

The most frequent value/range of a data set

The distribution around the sample mean The variability relative to the sample mean

The skewness relative to the sample mean The peakedness around the sample mean

Measures of correlation:

- sample covariance:
- sample coefficient

Tendency for high-high, low-low and high-low pairs in two data sets

of correlation : Normalized coefficient between -1 and +1 ETH Swiss Federal Institute of Technology

• Assume that we have a set of data (observations of road way traffic)

The simplest representation of the data is the one-dimensional scatter plot



	-		1	
Date	Dire	ction 1	Direc	nion 2
Dute	Unordered	Ordered	Unordered	Ordered
01.01	3087	3087	3677	3677
02.01	4664	3578	7357	4453
03.01	4164	3710	9323	4480
04.01	3710	3737	11748	4560
05.01	4029	3906	10256	4635
06.01	4323	4029	4453	4648
07.01	4041	4041	4815	4672
08.01	3737	4085	4757	4757
09.01	4103	4103	4672	4791
10.01	5457	4164	5401	4815
11.01	4563	4323	5688	4880
12.01	3906	4359	6308	4928
13.01	4419	4366	4946	4946
14.01	4359	4368	4635	5005
15.01	4667	4371	5100	5013
16.01	5098	4419	4791	5100
17.01	6551	4563	5235	5220
18.01	4371	4588	4560	5235
19.01	3578	4664	5729	5281
20.01	4366	4667	5005	5318
21.01	4368	4727	4480	5398
22.01	4588	4739	4880	5401
23.01	5001	4741	4928	5679
24.01	7118	5001	5398	5688
25.01	4727	5098	4648	5729
26.01	4085	5193	6183	6183
27.01	4741	5457	5220	6308
28.01	4739	5892	5013	7357
29.01	5193	6551	5281	9323
30.01	5892	7118	5318	10256
31.01	7974	7974	5679	11748

• Histograms

 $=\frac{1}{31}$ +00

The data are grouped into intervals

Data	Dire	ection 1	Direction 2	
Date	Unordered	Ordered	Unordered	Ordered
01.01	3087	3087	3677	3677
02.01	4664	3578	7357	4453
03.01	4164	3710	9323	4480
04.01	3710	3737	11748	4560
05.01	4029	3906	10256	4635
06.01	4323	4029	4453	4648
07.01	4041	4041	4815	4672
08.01	3737	4085	4757	4757
09.01	4103	4103	4672	4791
10.01	5457	4164	5401	4815
11.01	4563	4323	5688	4880
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15.01	4667	4371	5100	5013
16.01	5098	4419	4791	5100
17.01	6551	4563	5235	5220
18.01	4371	4588	4560	5235
19.01	3578	4664	5729	5281
20.01	4366	4667	5005	5318
21.01	4368	4727	4480	5398
22.01	4588	4739	4880	5401
23.01	5001	4741	4928	5679
24.01	7118	5001	5398	5688
25.01	4727	5098	4648	5729
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27.01	4741	5457	5220	6308
28.01	4739	5892	5013	7357
29.01	5193	6551	5281	9323
30.01	5892	7118	5318	10256
31.01	7974	7974	5679	11748

	Interval (Number of cars x 10 ² /day)	Interval Midpoint (Number of cars x 10 ² /day)	Number of observations	Frequency (%)	Cumulative frequency
	35-40	37.5	1	3.2258	0.0323
	40-45	42.5	2	6.4516	0.0968
	45-50	47.5	10	32.2581	0.4194
	50-55	52.5	9	29.0323	0.7097
	55-60	57.5	3	9.6774	0.8065
	60-65	62.5	2	6.4516	0.8710
	65-70	67.5	0	0.0000	0.8710
	70-75	72.5	1	3.2258	0.9032
	75-80	77.5	0	0.0000	0.9032
	80-85	82.5	0	0.0000	0.9032
•	85-90	87.5	0	0.0000	0.9032
	90-95	92.5	1	3.2258	0.9355
	95-100	97.5	0	0.0000	0.9355
	100-105	102.5	1	3.2258	0.9677
	105-110	107.5	0	0.0000	0.9677
	110-115	112.5	0	0.0000	0.9677
	115 1-20	117.5	1	3.2258	1.0000
	115 1-20	117.3	1	5.2258	1.0000

 $\Sigma = 31$

Histograms



• Histograms

The grouped data are plotted

Interval (Number of cars $*10^2$)	Interval Midpoint (Number of cars *10 ²)	Number of observations	Frequenc [%]	Cumulative frequency
30-35	32.5	0	0.0000	0.0000
35-40	37.5	1	3.2258	0.0323
40-45	42.5	2	6.4516	0.0968
45-50	47.5	10	32.2581	0.4194
50-55	52.5	9	29.0323	0.7097
55-60	57.5	3	9.6774	0.8065
60-65	62.5	2	6.4516	0.8710
65-70	67.5	0	0.0000	0.8710
70-75	72.5	1	3.2258	0.9032
75-80	77.5	0	0.0000	0.9032
80-85	82.5	0	0.0000	0.9032
85-90	87.5	0	0.0000	0.9032
90-95	92.5	1	3.2258	0.9355
95-100	97.5	0	0.0000	0.9355
100-105	102.5	1	3.2258	0.9677
105-110	107.5	0	0.0000	0.9677
110-115	112.5	1	3.2258	1.0000

Cumulative frequency distribution

Number of cars x 10²

• Histograms

The number of intervals selected will influence the information maintained

No general rule can be given but some suggest the following

k = 1 + 3.3 logn

k: number of intervalsn: number of data

For the traffic flow data set: $k=1+3.3 \log 31=5.92=6$



• Histograms

The number of intervals selected will influence the information maintained

k=17



Number of cars x 102

k=6



Quantile plots

Definition : the Q-quantile corresponds to the value in a data set which is exceeded by 100% - Q x 100% of the data

e.g. the 0.75 quantile is exceeded by 100% - 0.75 x 100% = 25% of the data

Quantile plots are generated by plotting the data against their quantile values







r : Inter-quartile range (50% of data)

Tukey Box plots (traffic data)

Statistic	
Lower adjacent value	
Lower quartile	
Median	
Upper quartile	
Upper adjacent value	
Outside values	
Direction 1	Direction 2
Direction 1 3087	Direction 2 3677
Direction 1 3087 4085	Direction 2 3677 4757
Direction 1 3087 4085 4419	Direction 2 3677 4757 5100
Direction 1 3087 4085 4419 5001	Direction 2 3677 4757 5100 5688
Direction 1 3087 4085 4419 5001 5892	Direction 2 3677 4757 5100 5688 6308
Direction 1 3087 4085 4419 5001 5892 6551	Direction 2 3677 4757 5100 5688 6308 7357
Direction 1 3087 4085 4419 5001 5892 6551 7118	Direction 2 3677 4757 5100 5688 6308 7357 9323
Direction 1 3087 4085 4419 5001 5892 6551 7118 7974	Direction 2 3677 4757 5100 5688 6308 7357 9323 10256



• Q-Q plots

Q-Q plots are produced to represent and compare 2 data sets

Data points of the two data sets with the same quantile values are plotted against each other



Number of cars in direction 1



• Mean vs. difference plots

Mean vs. difference plots are produced to represent and compare 2 data sets

$$(y_i + x_i)/2$$

is plotted against

 $y_i - x_i$





• Summary

One-dimensional scatter plots : illustrate the range and distribution of a data sets along one axis, indicate symmetry. illustrate how the data are distributed Histograms: over the range of data, indicate mode and symmetry. Illustrate median, distribution and Quantile plots: symmetry Tukey - Box plots: Illustrate median, upper/lower quartiles, symmetry and distribution Q-Q plots: Compare two data set, relative shapes Mean vs. difference plots: Compare two data sets, relative shapes **ETH** Swiss Federal Institute of Technology

Statistics and Probability Theory in

Civil, Surveying and Environmental Engineering

Prof. Dr. Michael Havbro Faber Swiss Federal Institute of Technology ETH Zürich, Switzerland



Contents of Today's Lecture

- Overview of Uncertainty Modelling
- Uncertainties in Engineering Problems
- Random Variables
 - discrete cumulative distribution and probability density functions
 - continuous cumulative distribution and probability density functions
 - characterization of random variables
 - moments of random variables
 - the expectation and the variance operator

Overview of Uncertainty Modelling

Why uncertainty modelling



Different types of uncertainties influence decision making

- Inherent natural variability aleatory uncertainty
 - result of throwing dices
 - variations in material properties
 - variations of wind loads
 - variations in rain fall
- Model uncertainty epistemic uncertainty
 - lack of knowledge (future developments)
 - inadequate/imprecise models (simplistic physical modelling)
- Statistical uncertainties epistemic uncertainty
 - sparse information/small number of data

- Consider as an example a dike structure
 - the design (height) of the dike will be determining the frequency of floods
 - if exact models are available for the prediction of future water levels and our knowledge about the input parameters is perfect then we can calculate the frequency of floods
 (per year) - a deterministic world !
 - even if the world would be deterministic we would not have perfect information about it - so we might as well consider the world as random

In principle the so-called

inherent physical uncertainty (aleatory - Type I)

is the uncertainty caused by the fact that the world is random, however, another pragmatic viewpoint is to define this type of uncertainty as

any uncertainty which cannot be reduced by means of collection of additional information

the uncertainty which can be reduced is then the

model and statistical uncertainties (epistemic - Type II)







The relative contribution of aleatory and epistemic uncertainty to the prediction of future water levels is thus influenced directly by the applied models

refining a model might reduce the epistemic uncertainty – but in general also changes the contribution of aleatory uncertainty

the uncertainty structure of a problem can thus be said to be scale dependent !





The uncertainty structure changes also as function of time - is thus time dependent !
- Probability density and cumulative distribution functions
 - A random variable is denoted with capital letters : X
 - A realization of a random variable is denoted with small letters : x
 - We distinguish between
 - continuous random variables : can take any value

•

- discrete random variables

- can take any value in a given range
- can take only discrete values



• Probability density and cumulative distribution functions

The probability that the outcome of a discrete random variable X is smaller than x is denoted the cumulative distribution function

$$P_X(x) = \sum_{x_i < x} p_X(x_i)$$

The *probability density function* for a discrete random variable is defined by

$$p_X(x_i) = P(X = x_i)$$



• Probability density and cumulative distribution functions

 $F_{x}(x)$ Α The probability that the outcome of a continuous random variable X is smaller than x is denoted the cumulative distribution function $F_{X}(x) = P(X < x)$ $f_{\chi}(x)$ В The *probability density function* for a continuous random variable is defined by $f_X(x) = \frac{\partial F_X(x)}{\partial x}$ х **Integral of this must equal 1 ETH** Swiss Federal Institute of Technology

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• Moments of random variables and the expectation operator

Probability distributions (cumulative distribution function and probability density function) can be described in terms of their paramaters \mathbf{p} or their moments

Often we write

$$F_X(x,\mathbf{p}) \qquad f_X(x,\mathbf{p})$$

Parameters

The parameters can be related to the moments and visa versa



• Moments of random variables and the expectation operator

The i'th moment m_i for a continuous random variable X is defined through

$$m_i = \int_{-\infty}^{\infty} x^i f_X(x) dx$$

The expected value E[X] of a continuous random variable X is defined accordingly as the first moment

$$\mu_{X} = E[X] = \int_{-\infty}^{\infty} x f_{X}(x) dx$$

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 $-\infty$

• Moments of random variables and the expectation operator

The i'th moment m_i for a discrete random variable X is defined through

$$m_i = \sum_{j=1}^n x_j^i p_X(x_j)$$

The expected value E[X] of a discrete random variable X is defined accordingly as the first moment

$$\mu_X = E[X] = \sum_{j=1}^n x_j p_X(x_j)$$

• Moments of random variables and the expectation operator

The expected value (or mean value) of a random variable can be understood as the *center of gravity* of the probability density function of the random variable !



Moments of random variables and the expectation operator

The variance σ_X^2 of a continuous random variable is defined as the second central moment i.e. for a continuous random variable X we have

$$\sigma_X^2 = Var[X] = E[(X - \mu_X)^2] = \int_{-\infty}^{\infty} (x - \mu_X)^2 f_X(x) dx$$

t
Variance
Variance
Mean value

for a discrete random variable we have correspondingly

$$\sigma_X^2 = Var[X] = \sum_{j=1}^n (x_j - \mu_X)^2 p_X(x_j)$$

• Moments of random variables and the expectation operator

The ratio between the standard deviation and the expected value of a random variable is called the *Coefficient of Variation CoV* and is defined as

$$CoV[X] = \frac{\sigma_X}{\mu_X}$$

Dimensionless

a useful characteristic to indicate the variability of the random variable around its expected value



• Example - uniformly distributed random variable

probability density and cumulative distribution functions

$$f_{x}(x) = \begin{cases} 0, & x < a \\ \frac{1}{b-a}, & a \le x \le b \\ 0, & b < x \end{cases}$$

$$\begin{array}{c|c} & & \\ & & \\ & & \\ & & \\ & a & b \end{array} \xrightarrow{} x$$

 $f_{\mathcal{X}}(x)$

$$F_{X}(x) = \begin{cases} 0, & x < a \\ \int_{a}^{x} f_{X}(y) dy = \int_{a}^{x} \frac{1}{b-a} dy = \frac{(x-a)}{(b-a)}, & a \le x \le b \\ 1, & b < x \end{cases}$$



• Example - uniformly distributed random variable



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Contents of Todays Lecture

- Overview of Uncertainty Modeling
- Random Variables
 - properties of the expectation operator
 - random vectors and joint moments
 - conditional distributions and conditional moments
 - the probability distribution for the sum of two random variables
 - the probability distribution for functions of random variables

Overview of Uncertainty Modeling

Random variables and their characteristics





• Properties of the expectation operator

The expectation operator facilitates that we can assess the expected value and the variance of a random variable

By understanding how the expectation operator works we will be able to assess the expected value and the variance of functions of random variables

This is useful if we want to analyze engineering models involving one or more random variables in regard to their expected values and their variances

E.g.: Duration of a construction process as a function of the duration of its individual processes

Properties of the expectation operator

The expectation operator possesses the following properties:

$$E[c] = c$$

$$E[cX] = cE[X]$$

$$E[a + bX] = a + bE[X]$$

$$E[g_1(X) + g_2(X)] = E[g_1(X)] + E[g_2(X)]$$

Properties of the expectation operator

The variance can thus be written as:

$$Var[X] = E[(X - \mu_X)^2]$$

= $E[X^2 + \mu_X^2 - 2\mu_X X]$
= $\mu_X^2 + E[X^2] - 2\mu_X E[X]$
= $\mu_X^2 + E[X^2] - 2\mu_X^2 = E[X^2] - \mu_X^2$



Properties of the expectation operator

Furthermore there is

Var[c] = 0 $Var[cX] = c^{2}Var[X]$ $Var[a+bX] = b^{2}Var[X]$

E[c] = c E[cX] = cE[X] E[a + bX] = a + bE[X] $E[g_1(X) + g_2(X)] = E[g_1(X)] + E[g_2(X)]$



• Properties of the expectation operator

From the result

$$Var[X] = E[(X - \mu_X)^2] = E[X^2 + \mu_X^2 - 2\mu_X X] = E[X^2] - \mu_X^2$$

it is seen that there in general is $E[g(X)] \neq g(E[X])$

 $E[g(X)] \ge g(E[X])$ for convex functions - Jensen's inequality ! **f** Equality only for linear functions

Random vectors and joint moments

Often we are dealing with models involving not only one random variable but several random variables

- These random variables can be collected in a vector
- In general the components of the vector are dependent
- E.g. Rainfall and water level

It is thus necessary that we establish probabilistic models which include this dependency - we can do this through the joint cumulative distributions and the joint moments.

Random vectors and joint moments

Now we consider not just one continuous random variable but a vector of continuous random variables

$$\mathbf{X} = \left(X_1, X_2, \dots, X_n\right)^T$$

The joint cumulative distribution function is given by $F_{\mathbf{X}}(\mathbf{x}) = P(X_1 \le x_1 \cap X_2 \le x_2 \cap \ldots \cap X_n \le x_n)$

and the joint probability density function is given by

$$f_{\mathbf{X}}(\mathbf{x}) = \frac{\partial^{n}}{\partial z_{1} \partial z_{2} \dots \partial z_{n}} F_{\mathbf{X}}(\mathbf{x})$$

• Random vectors and joint moments

Consider the two dimensional discrete probability density function:

х,у	p(x,y)
1,10	0.033
1,20	0.067
1,30	0.033
1,40	0.033
2,10	0.067
2,20	0.100
2,30	0.067
2,40	0.033
3,10	0.067
3,20	0.133
3,30	0.100
3,40	0.067
4,10	0.033
4,20	0.067
4,30	0.067
4,40	0.033





Random vectors and joint moments

The marginal probability density function of a random variable X_i is defined by

$$f_{X_i}(x) = \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} (n-1 \text{ fold}) f_{\mathbf{X}}(\mathbf{x}) dx_1 ... dx_{i-1} dx_{i+1} ... dx_n$$



Random vectors and joint moments

Consider the two dimensional discrete probability density function:

x,y	p(x,y)
1,10	0.033
1,20	0.067
1,30	0.033
1,40	0.033
2,10	0.067
2,20	0.100
2,30	0.067
2,40	0.033
3,10	0.067
3,20	0.133
3,30	0.100
3,40	0.067
4,10	0.033
4,20	0.067
4,30	0.067
4,40	0.033



Discrete joint density



Marginal density for x

Random vectors and joint moments

The covariance between the i'th and the j'th component of the random vector of continuous random variables is defined as the *joint central moment* i.e. by

$$C_{X_{i}X_{j}} = E\left[(X_{i} - \mu_{X_{i}})(X_{j} - \mu_{X_{j}})\right] = \int_{-\infty-\infty}^{\infty} \int_{-\infty-\infty}^{\infty} (x_{i} - \mu_{X_{i}})(x_{j} - \mu_{X_{j}})f_{X_{i}X_{j}}(x_{i}, x_{j})dx_{i}dx_{j}$$
$$C_{X_{i}X_{i}} = Var[X_{i}]$$

From where we see that for i = j we get the variance for X_i

Correlation coefficient
$$\rho_{X_i X_j} = \frac{C_{X_i X_j}}{\sigma_{X_i} \sigma_{X_j}} \qquad \rho_{X_i X_i} = 1$$

Random vectors and joint moments

The expected value and the variance of a linear function

$$Y = a_0 + \sum_{i=1}^n a_i X_i$$

are given by

$$E[Y] = a_0 + \sum_{i=1}^n a_i E[X_i]$$
$$Var[Y] = \sum_{i=1}^n a_i^2 Var[X_i] + 2\left(\sum_{\substack{i,j=1\\i\neq j}}^n a_i a_j C_{X_i X_j}\right)$$



• Conditional distributions and conditional moments

Some times it is useful to be able to assess the probability of an event given that we know something about one of the random variables which are used to define the event

E.g. assume we want to calculate the probability that a project will be delayed under the condition that one of the processes will exceed its planned duration by 50%.



• Conditional distributions and conditional moments

The conditional probability density function for the random variable X_1 given the outcome of the random variable X_2 is given by $f_{X_1|X_2}(x_1|x_2) = \frac{f_{X_1,X_2}(x_1,x_2)}{f_{X_2}(x_2)}$

where if X_1 and X_2 are independent

$$f_{X_1|X_2}(x_1|x_2) = f_{X_1}(x_1)$$

The conditional cumulative distribution function is obtained by integration as x_{l}

$$F_{X_1|X_2}(x_1|x_2) = \frac{\int_{-\infty}^{\infty} f_{X_1,X_2}(z,x_2) dz}{f_{X_2}(x_2)}$$

Conditional distributions and conditional moments

The un-conditional cumulative distribution function for the random variable X_1 can be derived from the conditional comulative distribution function by use of the total probability theorem

$$F_{X_1}(x_1) = \int_{-\infty}^{\infty} F_{X_1|X_2}(x_1|x_2) f_{X_2}(x_2) dx_2$$

The conditional expected value is defined by

$$\mu_{X_1|X_2} = E\left[X_1 | X_2 = x_2\right] = \int_{-\infty}^{\infty} x_1 f_{X_1|X_2}(x|x_2) dx_1$$

• In many cases we are interested in assessing the probabilites of functions of random variables

The functions are useful for describing the events we are interested in - they are our engineering models.

A simple case is the sum of two random variables – it is useful to derive the cumulative distribution function for such a sum.

A more general case concerns monotonic functions of random variables – we will also derive the cumulative distribution for this case.



• The cumulative distribution function for the sum of two random variables

Consider the sum $Y = X_1 + X_2$

and assume that we have $f_{X_1,X_2}(x_1,x_2)$

First we derive the density function for $Y = x_1 + X_2$

assuming that
$$X_1$$
 is given i.e. $f_{X_2|X_1}(x_2|x_1) = \frac{f_{X_1,X_2}(x_1,x_2)}{f_{X_1}(x_1)}$
 $f_{Y|X_1}(y|x_1) = f_{X_2|X_1}(y-x_1|x_1)$

and we get $f_{Y,X_1}(y,x_1) = f_{X_2|X_1}(y-x_1|x_1)f_{X_1}(x_1) = f_{X_2,X_1}(y-x_1,x_1)$

• The cumulative distribution function for the sum of two random variables

The marginal probability density function for Y is now achieved by integrating out over X_1 , i.e.

$$f_{Y}(y) = \int_{-\infty}^{\infty} f_{X_{2},X_{1}}(y - x_{1}, x_{1}) dx_{1}$$

 ∞

For the case where X_1 and X_2 are independent we get the so-called *convolution integral*

$$f_{Y}(y) = \int_{-\infty}^{\infty} f_{X_{2}}(y - x_{1}) f_{X_{1}}(x_{1}) dx_{1}$$

• The cumulative distribution function for functions of random variables

Consider the more general problem of deriving the cumulative distribution function for a function of a random variables i.e. Y = g(X) where the probability distribution function of X is given as $F_X(x)$

If g(x) is monotonically increasing and represents a one-to-one mapping, a realization of Y is only smaller than y_0 if the realization of X is smaller than x_0 where $x_0 = g^{-1}(y_0)$ $F_Y(y) = P(Y \le y) = P(X \le g^{-1}(y))$

The cumulative distribution function for Y is then given by $F_{Y}(y) = F_{X}(g^{-1}(y))$

The cumulative distribution function for functions of random • variables

starting now with $F_{y}(y) = F_{x}(g^{-1}(y))$

we have
$$f_Y(y) = \frac{\partial F_X(g^{-1}(y))}{\partial y}$$

• The cumulative distribution function for functions of random variables

In case the function g(x) is monotonically decreasing, a realization of Y is only smaller than y_0 if the realization of X is larger than x_0 , and in this case we have to change the sign i.e. $F_Y(y) = -F_X(g^{-1}(y))$

yielding
$$f_Y(y) = -\frac{\partial x}{\partial y} f_X(x)$$

In the general case – for monotonically increasing or decreasing functions there is thus $f_Y(y) = \left| \frac{\partial x}{\partial y} \right| f_X(x)$

• The cumulative distribution function for functions of random variables

For the case where the components of a random vector $\mathbf{Y} = (Y_1, Y_2, .., Y_n)^T$ can be given as one-to-one mappings of monotonically increasing or decreasing functions $g_i, i = 1, 2, .., n$ of the components of a random vector $\mathbf{X} = (X_1, X_2, .., X_n)^T$

in the form: $Y_i = g_i(\mathbf{X})$

there is $f_{\mathbf{Y}}(\mathbf{y}) = \left| \mathbf{J} \right| f_{\mathbf{X}}(\mathbf{x})$

with $|\mathbf{J}|$ being the absolute value of the determinant o




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Overview of Uncertainty Modeling

• Random variables and their characteristics





Overview of Uncertainty Modeling

Random variables and their characteristics

Design of rock-fall galleries









Overview of Uncertainty Modeling

Random variables and their characteristics



 Engineering problems – also those involving uncertainty are very often specific – unique !

Being able to solve such problems requires

- basic tools (physical, mathematical, natural sciences, human sciences, engineering,...)
- innovation (being able to identify ways of solving problems)
- training !

Training is important because it provides experience.

By training we start to recognize patterns !

 Pattern recognition helps to identify:

the usefulness of solution strategies from previous problems

the potential of the available tools in a given context







Random variables and their characteristics

The expectation operator

E[c] = c E[cX] = cE[X] E[a + bX] = a + bE[X] $E[g_1(X) + g_2(X)] = E[g_1(X)] + E[g_2(X)]$

The variance operator

Var[c] = 0 $Var[cX] = c^{2}Var[X]$ $Var[a+bX] = b^{2}Var[X]$

Jointly distributed random variables



$$F_{\mathbf{X}}(\mathbf{x}) = P(X_1 \le x_1 \cap X_2 \le x_2 \cap \ldots \cap X_n \le x_n)$$

• Random variables and their characteristics

Functions of random variables

- sum of two random variables

$$Y = X_1 + X_2$$

$$f_Y(y) = \int_{-\infty}^{\infty} f_{X_2}(y - x_1) f_{X_1}(x_1) dx_1$$

- non-linear function of random variables

$$Y = g(X)$$
$$f_Y(y) = \left| \frac{\partial x}{\partial y} \right| f_X(x)$$

• Random variables and their characteristics

Functions of random variables

$$\mathbf{Y} = (Y_1, Y_2, \dots Y_n)^T$$

$$Y_i = g_i(\mathbf{X}), \qquad X_i = f_i(\mathbf{Y})$$

$$f_{\mathbf{Y}}(\mathbf{y}) = \left| \mathbf{J} \right| f_{\mathbf{X}}(\mathbf{x}) \qquad \mathbf{J} = \begin{bmatrix} \frac{\partial x_1}{\partial y_1} & \dots & \frac{\partial x_1}{\partial y_n} \\ \vdots & \ddots & \vdots \\ \frac{\partial x_n}{\partial y_1} & \dots & \frac{\partial x_n}{\partial y_n} \end{bmatrix}$$

Contents of Todays Lecture

- Random variables
 - The Central Limit Theorem
 - The Normal distribution
 - The Log-Normal distribution
- Stochastic Processes and Extremes
 - Random sequences (Bernoulli trials)
 - Binomial distribution
 - Geometric distribution

• The Central Limit Theorem states:

The probability distribution function of a sum of a number of random variables approaches the Normal (Gaussian) distribution as the number becomes large

$$Y = X_1 + X_2 + \dots + X_n$$
$$f_X(x) = \frac{1}{\sigma\sqrt{2\pi}} exp\left(-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^2\right)$$
$$F_X(x) = \frac{1}{\sigma\sqrt{2\pi}} \int_{-\infty}^x exp\left(-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^2\right) dx$$





• The Central Limit Theorem

Conditions for the validity of the theorem: $Y = X_1 + X_2 + ... + X_n$

The sum should not be dominated by one or a few components

The statistical dependency between components should not be strong

No requirements to the type of distribution of the components

If the components have skew distributions the number increases

- Illustration:
 - A structural member is measured using a ruler.
 - The ruler has limited length (2 m).
 - The smallest unit on the ruler is 1 mm.

All measurements are rounded to the closest unit on the ruler.

Each measurement is subject to a measurement uncertainty uniformly distributed in the range of +/-0.5 mm.

We now consider the accumulated error associated with measurements over lengths

- up to 2 m
- between 2 and 4 m
- between 6 and 8 m
- between 14 and 16 m

(one measurement) (two measurements) (four measurements) (eight measurements)

• Illustration:





250

200





N=2

0 °į

Error

N=8

۰ ⁽۶ ۲



• The Normal distribution

The analytical form of the Normal distribution may be derived by repeated use of the result regarding the probability density function for the sum of two random variables

The Normal distribution is very frequently applied in engineering modeling when a random quantity can be assumed to be composed as a sum of a number of individual contributions: $X_i, i=1,2,..,n$

A linear combination S of *n* Normal distributed random variables $S = a_0 + \sum_{i=1}^{n} a_i X_i$ is thus also a Normal distributed random variable

The Normal distribution

The Normal distribution also results from other considerations

The distribution of energy in an isolated system If the particles represent gas molecules at normal temperatures

If the particles represent gas molecules at normal temperatures inside a closed container, which of the illustrated configurations came first?



If you tossed bricks off a truck, which kind of pile of bricks would you more likely produce?





• The Normal distribution

The accumulation of random movements



• The Normal distribution:

In the case where the mean value is equal to zero and the standard deviation is equal to 1 the random variable is said to be *standardized*.



• The Normal distribution:

In the case where the mean value is equal to zero and the standard deviation is equal to 1 the random variable is said to be *standardized*.

Standard normal







When the logarithm of a random variable X i.e.

 $Y = In(X), \quad Y : N(\mu_y, \sigma_y)$

is Normal distributed the random variable X is said to be Log-Normal distributed

 $X : LN(\lambda,\zeta)$

$$f_X(x) = \frac{1}{x\zeta\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{\ln(x) - \lambda}{\zeta}\right)^2\right) \qquad \qquad \mu_X = \exp\left(\lambda + \frac{\zeta^2}{2}\right)$$

$$F_X(x) = \Phi\left(\frac{\ln(x) - \lambda}{\zeta}\right) \qquad \qquad \sigma_X = \exp\left(\lambda + \frac{\zeta^2}{2}\right) \sqrt{\exp(\zeta^2) - 1}$$

Where the Normal distribution follows from the sum of random variables - Central Limit Theorem

the Log-Normal distribution follows from the product of random variables

$$\ln(X_1 \cdot X_2 \cdots X_n) = \ln(\prod_{i=1}^n X_i) = \sum_{i=1}^n \ln(X_i)$$



The Log-Normal distribution has the useful property that if

$$P = \prod_{i=1}^{n} Y_i^{a_i}$$

and all Y_i are independent Log-Normal distributed random variables with parameters ζ_i , λ_i and $\varepsilon_i = 0$ then P is also Log-Normal with parameters

$$\lambda_P = \sum_{i=1}^n a_i \lambda_i \qquad \qquad \zeta_P^2 = \sum_{i=1}^n a_i^2 \zeta_i^2 \qquad f_P(p) = \frac{1}{p \zeta_P \sqrt{2\pi}} \exp\left(-\frac{1}{2} \left(\frac{\ln(p) - \lambda_P}{\zeta_P}\right)^2\right)$$



The Log-Normal distribution is often used to model

- uncertain parameters which cannot have negative realizations
- fatigue lives
- steel and concrete resistance
- daily river flows
- whenever a random variable results as a product of several random variables



Concrete compression strength

Probability of value lower than 25 MPa

$$F_X(25) = \Phi\left(\frac{\ln(25) - 3.48}{0.12}\right) = 0.018$$



There exist a large number of different probability density and cumulative distribution functions:

Uniform Normal Log-normal Exponential Beta Gamma

...

...

Distribution type	Parameters	Moments
Uniform, $a \le x \le b$ $f_x(x) = \frac{1}{b-a}$ $F_x(x) = \frac{x-a}{b-a}$	a b	$\mu = \frac{a+b}{2}$ $\sigma = \frac{b-a}{\sqrt{12}}$
Normal $f_{x}(x) = \frac{1}{\sigma\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^{2}\right)$	μ $\sigma > 0$	μ σ
$F_{X}(x) = \frac{1}{\sigma\sqrt{2\pi}} \int_{-\infty}^{\infty} \exp\left(-\frac{1}{2}\left(\frac{t-\mu}{\sigma}\right)\right) dt$ Shifted Lognormal, $x > \varepsilon$ $f_{X}(x) = \frac{1}{(x-\varepsilon)\zeta\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{\ln(x-\varepsilon)-\lambda}{\zeta}\right)^{2}\right)$ $F_{X}(x) = \Phi\left(\frac{\ln(x-\varepsilon)-\lambda}{\zeta}\right)$	λ $\zeta > 0$ ε	$\mu = \varepsilon + \exp\left(\lambda + \frac{\zeta^2}{2}\right)$ $\sigma = \exp\left(\lambda + \frac{\zeta^2}{2}\right)\sqrt{\exp(\zeta^2) - \frac{\zeta^2}{2}}$
Shifted Exponential, $x \ge \varepsilon$ $f_x(x) = \lambda \exp(-\lambda(x-\varepsilon))$ $F_x(x) = 1 - \exp(-\lambda(x-\varepsilon))$	ε λ>0	$\mu = \varepsilon + \frac{1}{\lambda}$ $\sigma = \frac{1}{\lambda}$
Gamma, $x \ge 0$ $f_{x}(x) = \frac{b^{p}}{\Gamma(p)} \exp(-bx)x^{p-1}$ $F_{x}(x) = \frac{\Gamma(bx, p)}{\Gamma(p)}$	<i>p</i> > 0 <i>b</i> > 0	$\mu = \frac{p}{b}$ $\sigma = \frac{\sqrt{p}}{b}$

Small Example 1

We remember the convolution integral which we used for establishing the probability density function for the sum of two random variables:

$$Y = X_1 + X_2$$

$$f_Y(y) = \int_{-\infty}^{\infty} f_{X_2}(y - x_1) f_{X_1}(x_1) dx_1$$

Let us see how easily this works for two uniformly distributed random variables:





Small Example 1

0.3

0.25

0.2

0.15

0.1

0.05

0

0

a=4, b=8

c=2, d=6

5

Assuming that the two random variables are independent we can write the convolution integral as:

$$f_{Y}(y) = \int_{-\infty}^{\infty} f_{X_{2}}(y - x_{1}) f_{X_{1}}(x_{1}) dx_{1}$$

$$= \frac{1}{(b - a)(d - c)} \int_{a}^{b} (y - x_{1} \in [c; d]) dx_{1}$$

$$= \frac{1}{(b - a)(d - c)} [x_{1}]_{\max(c, y - a)}^{\min(d, y - b)}, \quad a + c \le y \le b + d$$





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- Random quantities may be "time variant" in the sense that they take new values at different times or at new trials.
 - If the new realizations occur at discrete times and have discrete values the random quantity is called a random sequence
 - failure events, traffic congestions,...

- If the new realizations occur continuously in time and take continues values the random quantity is called a random process or stochastic process

wind velocity, wave heights,...

Random sequences

A sequence of experiments with only two possible and mutually exclusive outcomes is called a Bernoulli trial
Typically the outcomes of Bernoulli trials are denoted successes or failures

If the probability of success in one trial is constant and equal to p the probability density of Y successes in n trials, i.e. $p_Y(y)$ is given by:

$$p_{y}(y) = \binom{n}{y} p^{y} (1-p)^{n-y}, \quad y = 0,1,2...n \qquad \binom{n}{y} = \frac{n!}{y!(n-y)!}$$
Binomial probability
Binomial probability
density function

Random sequences

- A sequence of experiments with only two possible and mutually exclusive outcomes is called a Bernoulli trial

The Binomial cumulative distribution function then follows as:

$$P_{Y}(y) = \sum_{i=0}^{y} {\binom{y}{i}} p^{i} (1-p)^{n-i}, \qquad y = 0, 1, 2, \dots n$$



Random sequences

- A sequence of experiments with only two possible and mutually exclusive outcomes is called a Bernoulli trial

Illustration:



Binomial probability density function for n=5 and p=0.15 and

Small Example 2

We remember that we can establish the probability density function of a function of a random variable through:

$$Y = g(X)$$
$$f_{Y}(y) = \left| \frac{\partial x}{\partial y} \right| f_{X}(x)$$

Small Example 2

Let us see how easily this works:

$$Y = X^{2}$$

$$\Downarrow$$

$$X = \sqrt{Y}$$

$$f_{Y}(y) = \left|\frac{\partial x}{\partial y}\right| f_{X}(x)$$

$$f_{Y}(y) = \left|\frac{1}{2}y^{-\frac{1}{2}}\right| f_{Y}(\sqrt{y})$$

$$\frac{\partial x}{\partial y} = \frac{\partial \sqrt{y}}{\partial y} = \frac{1}{2} y^{-\frac{1}{2}}$$

$$f_Y(y) = \left| \frac{1}{2} y^{-\frac{1}{2}} \right| f_X(\sqrt{y})$$



• Random sequences

The expected value and the variance of a binomially distributed random variable Y is given by: E[Y] = np

$$Var[Y] = np(1-p)$$

Random sequences

The probability density function for the number of (independent) trials before the first success can be given as:

and the corresponding cumulative distribution function is thus

$$P_{N}(n) = \sum_{i=1}^{n} p(1-p)^{i-1} = 1 - (1-p)^{n}$$

Geometric cumulative distribution

Small Example 3

We remember that we could establish the probability density function of a vector of random variables Y which were given as functions of a vector of random variables X

$$\mathbf{Y} = (Y_1, Y_2, .., Y_n)^T$$

$$\mathbf{X} = (X_1, X_2, .., X_n)^T$$

$$Y_i = g_i(\mathbf{X})$$

$$X_i = f_i(\mathbf{Y})$$

$$f_{\mathbf{Y}}(\mathbf{y}) = \left| \mathbf{J} \right| f_{\mathbf{X}}(\mathbf{x})$$
$$\mathbf{J} = \begin{bmatrix} \frac{\partial x_1}{\partial y_1} & \dots & \frac{\partial x_1}{\partial y_n} \\ \vdots & \ddots & \vdots \\ \frac{\partial x_n}{\partial y_1} & \dots & \frac{\partial x_n}{\partial y_n} \end{bmatrix}$$
Small Example 3

Let us see how easily this approach can be applied for the following problem:

 $Y_1 = X_1 + X_2$ $X_1 = Y_1 - Y_2$ $Y_2 = X_2$ $X_2 = Y_2$

$$\mathbf{J} = \begin{bmatrix} 1 & -1 \\ 0 & 1 \end{bmatrix} \quad \det(\mathbf{J}) = 1 \times 1 - 0 \times 1 = 1 \Longrightarrow |\mathbf{J}| = 1$$

	$\frac{\partial x_1}{\partial y_1}$	••••	$\frac{\partial x_1}{\partial y_n}$
$\mathbf{J} =$	• •	••••	• •
	$\frac{\partial x_n}{\partial y_1}$	•••••	$\frac{\partial x_n}{\partial y_n}$

$$f_{\mathbf{Y}}(y_1, y_2) = f_{\mathbf{X}}(y_1 - y_2, y_2)$$



The median of the geometric distribution provides information in regard to how "long" we need to play a game with probability p of winning per time unit.

Time units might be

- tosses (dices)
- years (earthquakes)

The median is defined through

$$P_N(n) = 0.5 = 1 - (1 - p)^n$$

All we need to determine is n as a function of p

The median of the geometric distribution provides information in regard to how "long" we need to play a game with probability p of winning per time unit.

$$P_N(n) = 0.5 = 1 - (1 - p)^n$$

We take the natural logarithm on both sides and get:

$$\ln(0.5) = n \ln(1-p)$$

$$\Downarrow$$

 $0.7 \approx -n\ln(1-p)$

Now we use that the natural logarithm of $\ln(1-p) = -p + \frac{1}{2}p^2 - \frac{1}{3}p^3 + \dots = \sum_{k=1}^{\infty} (-1)^k \frac{p^k}{k}$ \Downarrow $\ln(1-p) \approx -p \text{ for small } p$ $\blacksquare Swiss Federal Institute of Technology$

$$0.7 \approx np \Longrightarrow n = \frac{0.7}{p}$$

We can now apply this result:

50% chance of getting a 6 requires (n tosses): $n = 0.7 \times 6 = 4$ tosses

50% chance of getting two 6 (with 2 dices) requires: $n = 0.7 \times 36 = 25$ tosses

50% chance experiencing an earthquake with an annual probability of 0.001 requires (n years): $n = 0.7 \times 1000 = 700$ years



Random sequences

The expected value and the variance of a random variable with a *Geometrically* distributed random variable are given by:

$$E[N] = \frac{1}{p}$$
$$Var[N] = \frac{1-p}{p^2}$$

If p is the annual probability of e.g. an extreme earthquake E[N] is the return period of such earthquakes



Statistics and Probability Theory in

Civil, Surveying and Environmental Engineering

Prof. Dr. Michael Havbro Faber Swiss Federal Institute of Technology ETH Zurich, Switzerland



Contents of Todays Lecture

- Presentation on the result of the classroom assessment
- What is a random variable?
- The decision context!
- What are we doing today?
- Details will follow ©

- Let us consider a very simple structural engineering problem!
- We want to design a steel beam and assume based on experience that the design controlling load effect is the midspan bending moment *M*
 - the design variable being the moment of resistance *W* of the cross section
 - the load *p* and the yield stress s_y of the beam are associated with uncertainty $\leftarrow b$





Mid span cross-section

• The moment capacity of the cross-section *R_M* and the mid span moment *M* are calculated as:

 $R_M = W\sigma_y$

 $W = \frac{1}{6}bh^2$

- R_M moment capacity of cross section
- W moment of resistance
- σ_{v} yield stress of the steel



- M mid span moment
- p load
- *l* length of beam



Mid span cross-section





- We can now establish a design equation as:
 - $R_{M}(b,h) M \ge 0$ \Downarrow $W(b,h)\sigma_{y} \frac{1}{4}Pl \ge 0$ \Downarrow $\frac{1}{6}bh^{2}\sigma_{y} \frac{1}{4}Pl \ge 0$

The engineer must now select *W*, or rather *b* and *h* such that the design equation is fulfilled

But as *p* and σ_y are associated with uncertainty – she/he must take this uncertainty into account !





Mid span cross-section



- The uncertainty is accounted for by representing p and sy in the design equation as two random variables.
 - *P*: Normal distributed: $N(\mu_P, \sigma_P)$
 - Σ_{y} : Normal distributed: $N(\mu_{\Sigma_{y}}, \sigma_{\Sigma_{y}})$



The random variable *P* represents the random variability of the load *p* during a period of one year

The random variable S_y represents the random variability of the steel yield stress s_y - produced by an unknown steel producer.



- As the load and yield stress are uncertain the design equation cannot be fulfilled with certainty – independent on the choice of b and h.
- However, it can be fulfilled with probability !
- The beam can be designed such that the probability of failure is less or equal to a given number – the requirement to safety.



Let us assume that the load and yield stress are given as:

$$P: \qquad N(\mu_P, \sigma_P) = N(100 \text{kN}, 20 \text{kN})$$

 Σ_{y} : $N(\mu_{\Sigma_{y}}, \sigma_{\Sigma_{y}}) = N(370\text{mPa}, 15\text{mPa})$

we can now write the event of failure as:



let us further assume that *I*=5000mm and *b*=50mm

• Let us now determine *h* such that the annual probability of failure is equal to 10⁻³

 We have already learned that a linear combination of Normal distributed random variables is also Normal distributed

The expected value of S is equal to:

$$\mu_{s} = \mu_{\Sigma_{y}} - \frac{3}{2 \cdot 0.05 \cdot h^{2}} \mu_{p} \cdot 5$$

= 370 - $\frac{3}{2 \cdot 0.05 \cdot h^{2}} \mu_{p} \cdot 5 = 370 - \frac{150000}{h^{2}}$

The variance of *S* is equal to:

$$\sigma_{S}^{2} = \sigma_{\Sigma_{y}}^{2} + \left(\frac{3}{2 \cdot 0.05 \cdot h^{2}}\right)^{2} \sigma_{P}^{2}$$
$$= 15^{2} + \left(\frac{30}{h^{2}}\right)^{2} \cdot 20000^{2} = 225 + \frac{3.6 \cdot 10^{11}}{h^{4}}$$

The probability of failure is now easily determined from the standard Normal cumulative distribution function

$$P_f(h) = \Phi(\frac{0 - \mu_s(h)}{\sigma_s(h)})$$



Calculating the probability of failure as a function of h we get:



The height of the beam must thus be equal to 73mm!

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The decision context!

• Why uncertainty modeling?



- We have already introduced random variables as a means of representing uncertainties which we may quantify based on observations – related to time frames from which we have experience and observations!
- In many real problems of decision making we need to take into account what might happen in the far future – exceeding the time frames for which we have experience!
 - 475 year design earthquake!
 - 100 year storm/flood
 - 100 year maximum truck load
 - etc.. Thus we need to develop models which can support us in the modeling of extremes of uncertain/random phenomena !

- We have already introduced random variables as a means of representing uncertainties which we may quantify based on observations.
- Often we use random variables to represent uncertainties which do not vary in time:
 - Model uncertainties (lack of knowledge)
 - Statistical uncertainties (lack of data).
- Or we use such random variables to represent the random variations which can be observed within a given reference period.



In many engineering problems we need to be able to describe the random variations in time more specifically:

The occurrences of events at random discrete points in time (rock-fall, earthquakes, accidents, queues, failures, etc.)

- Poisson process, exponential and Gamma distribution

The random values of events occurring continuously in time (wind pressures, wave loads, temperatures, etc.)

- Continuous random processes (Normal process)



Discrete event of flood



Continuous stress variations due to waves



- However, we are also interested in modeling extreme events such as:
 - the maximum value of an uncertain quantity within a given reference period
 - extreme value distributions

the expected value of the time till the occurrence of an event exceeding a certain severity - return period

Extreme water level



Maximum wave load





- In summary we will look at:
 - Random sequences (Poisson process)
 - Waiting time between events (Exponential and Gamma distributions)
 - Continuous random processes (the Normal process)
 - Criteria for extrapolation of extremes (stationarity and ergodicity)
 - The maximum value within a reference period (extreme value distributions)
 - Expected value of the time till the occurrence of an event exceeding a certain severity (return period)

 The Poisson counting process is one of the most commonly applied families of probability distributions applied in reliability theory

The process N(t) denoting the number of events in a (time) interval $(t,t+\Delta t)$ is called a Poisson process if the following conditions are fulfilled:

- 1) the probability of one event in the interval $(t,t+\Delta t)$ is asymptotically proportional to Δt .
- 2) the probability of more than one event in the interval $(t, t + \Delta t)$ is a function of higher order of Δt for $\Delta t \rightarrow 0$.
- 3) events in disjoint intervals are mutually independent.

The Poisson process can be described completely by its intensity n(t)

$$\nu(t) = \lim_{\Delta t \to 0} \frac{1}{\Delta t} P(\text{one event in } [t, t + \Delta t[)]$$

if n(t) = constant, the Poisson process is said to be homogeneous, otherwise it is inhomogeneous.

The probability of *n* events in the time interval (0, *t*[is:

$$P_n(t) = \frac{\left(\int_0^t v(\tau)d\tau\right)^n}{n!} \exp\left(-\int_0^t v(\tau)d\tau\right)$$

$$P_n(t) = \frac{\nu t^n}{n!} \exp(-\nu t)$$

Homogeneous case !



 The mean value and variance of the random variable describing the number of events in a given time interval (0,t[are given as:

$$E[N(t)] = Var[N(t)] = \int_{0}^{t} V(\tau) d\tau$$

Inhomogeneous case !

$$E[N(t)] = Var[N(t)] = Vt$$
 Homogeneous case !



The Exponential distribution

The probability of no events in a given time interval (0,t[is often of special interest in engineering problems

- no severe storms in 10 years
- no failure of a structure in 100 years
- no earthquake next year

-

This probability is directly achieved as:

$$P_0(t) = \frac{\left(\int_0^t v(\tau)d\tau\right)^0}{0!} \exp\left(-\int_0^t v(\tau)d\tau\right)$$
$$= \exp\left(-\int_0^t v(\tau)d\tau\right)$$

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 $P_0(t) = \exp(-\nu t)$

Homogeneous case !

• The probability distribution function of the (waiting) time till the first event T_1 is now easily derived recognizing that the probability of $T_1 > t$ is equal to $P_o(t)$ we get:

$$F_{T_1}(t_1) = 1 - P_0(t_1)$$

= 1 - exp(- $\int_0^t v(\tau) d\tau$)

Homogeneous case ! $F_{T_1}(t_1) = 1 - \exp(-\nu t)$

Exponential cumulative distribution

Exponential probability density

$$\oint_{T_1}(t_1) = v \exp(-vt)$$

The Exponential probability density and cumulative distribution functions



v = 2





- The exponential distribution is frequently applied in the modeling of waiting times
 - time till failure
 - time till next earthquake
 - time till traffic accident

$$f_{T_1}(t_1) = v \exp(-vt)$$

-

The expected value and variance of an exponentially distributed random variable T_1 are:

$$E[T_1] = \sqrt{Var[T_1]} = 1/v$$

- Sometimes also the time *T* till the *n*'th event is of interest in engineering modeling:
 - repair events
 - flood events
 - arrival of cars at a roadway crossing

If $T_{i'}$ i=1,2,...n are independent exponentially distributed waiting times, then the sum *T* i.e.:

$$T=T_1+T_2+\ldots+T_{n-1}+T_n$$

follows a Gamma distribution:

$$f_T(t) = \frac{\nu(\nu t)^{(n-1)} \exp(-\nu t)}{(n-1)!}$$

This follows from repeated use of the result of the distribution of the sum of two random variables



The Gamma probability density function



Continuous random processes

A continuous random process is a random process which has realizations continuously over time and for which the realizations belong to a continuous sample space.



Realization of continuous scalar valued random process

Continuous random processes

The mean value of the possible realizations of a random process is given as:

$$\mu_X(t) = E[X(t)] = \int_{-\infty}^{\infty} x f_X(x;t) dx$$

Function of time !

The correlation between realizations at any two points in time is given as:

$$R_{XX}(t_1,t_2) = E[X(t_1)X(t_2)] = \int_{-\infty}^{\infty} \int_{-\infty}^{\infty} x_1 x_2 f_{XX}(x_1,x_2;t_1,t_2) dx_1 dx_2$$

Auto-correlation function – refers to a scalar valued random process

Continuous random processes

The auto-covariance function is defined as:

$$C_{XX}(t_1, t_2) = E[(X(t_1) - \mu_X(t_1))(X(t_2) - \mu_X(t_2))]$$

= $\int_{-\infty}^{\infty} \int_{-\infty}^{\infty} (x_1 - \mu_X(t_1)) (x_2 - \mu_X(t_2)) f_{XX}(x_1, x_2; t_1, t_2) dx_1 dx_2$

for $t_1 = t_2 = t$ the auto-covariance function becomes the covariance function:

$$\sigma_X^2(t) = C_{XX}(t,t) = R_{XX}(t,t) - \mu_X^2(t)$$

$\sigma_{X}(t)$ Standard deviation function

Continuous random processes

A vector valued random process is a random process with two or more components:

$$\mathbf{X}(t) = (X_1(t), X_2(t), ..., X_n(t))^T$$

with covariance functions:

$$\begin{split} C_{X_i X_j}(t_1, t_2) &= & i = j \quad \text{auto-covariance functions} \\ E \Big[(X_i(t_1) - \mu_{X_i}(t_1))(X_j(t_2) - \mu_{X_j}(t_2)) \Big] & i \neq j \quad \text{cross-covariance functions} \end{split}$$

The correlation coefficient function is defined as:

$$\rho \Big[X_i(t_1), X_j(t_2) \Big] = \frac{C_{X_i X_j}(t_1, t_2)}{\sigma_{X_i}(t_1) \cdot \sigma_{X_j}(t_2)}$$

Normal or Gauss process

A random process X(t) is said to be Normal if:

For any set; $X(t_1), X(t_2), ..., X(t_j)$

the joint probability distributions of $X(t_1)$, $X(t_2)$,..., $X(t_i)$

is the Normal distribution.



Stationarity and ergodicity

A random process is said to be *strictly stationary* if all its moments are invariant to a shift in time.

A random process is said to be *weakly stationary* if the first two moments i.e. the mean value function and the auto-correlation function are invariant to a shift in time

$$\mu_{X}(t) = cst$$

$$R_{XX}(t_{1}, t_{2}) = f(t_{2} - t_{1})$$
Weakly stationary
- Stationarity and ergodicity
 - A random process is said to be *strictly ergodic* if it is strictly stationary and in addition all its moments may be determined on the basis of one realization of the process.
 - A random process is said to be *weakly ergodic* if it is weakly stationary *and in addition* its first two moments may be determined on the basis of one realization of the process.
- The assumptions in regard to stationarity and ergodicity are often very useful in engineering applications.
 - If a random process is ergodic we can extrapolate probabilistic models of extreme events within short reference periods to any longer reference period.

In engineering we are often interested in extreme values i.e. the smallest or the largest value of a certain quantity within a certain time interval e.g.:

The largest earthquake in 1 year

The highest wave in a winter season

The largest rainfall in 100 years



We could also be interested in the smallest or the largest value of a certain quantity within a certain volume or area unit e.g.:

The largest concentration of pesticides in a volume of soil

The weakest link in a chain

The smallest thickness of concrete cover





Observed monthly extremes

Observed annual extremes

Observed 5-year extremes

If the extremes within the period T of an ergodic random process X(t) are independent and follow the distribution:

 $F_{X,T}^{\max}(x)$

Then the extremes of the same process within the period:

$$n \cdot T$$

will follow the distribution:

$$F_{X,nT}^{\max}(x) = \left(F_{X,T}^{\max}(x)\right)^n$$



Extreme Type I – Gumbel Max

When the upper tail of the probability density function falls off exponentially (exponential, Normal and Gamma distribution) then the maximum in the time interval *T* is said to be Type I extreme distributed

$$f_{X,T}^{\max}(x) = \alpha \exp(-\alpha (x-u) - \exp(-\alpha (x-u)))$$

$$F_{X,T}^{\max}(x) = \exp(-\exp(-\alpha(x-u)))$$

$$\mu_{X_T^{\max}} = u + \frac{\gamma}{\alpha} = u + \frac{0.577216}{\alpha}$$

 $\sigma_{X_T^{\max}} = \frac{\pi}{\alpha\sqrt{6}}$

For increasing time intervals the variance is constant but mean value increases as: $\mu_{X_{nT}^{\max}} = \mu_{X_{T}^{\max}} + \frac{\sqrt{6}}{\pi} \sigma_{X_{T}^{\max}} \ln(n)$

Extreme Type II – Frechet Max

When a probability density function is downwards limited at zero and upwards falls off in the form

$$F_X(x) = 1 - \beta \left(\frac{1}{x}\right)^k$$

then the maximum in the time interval *T* is said to be Type II extreme distributed

$$F_{X,T}^{\max}(x) = \exp\left(-\left(\frac{u}{x}\right)^k\right)$$
$$f_{X,T}^{\max}(x) = \frac{k}{u}\left(\frac{u}{x}\right)^{k+1}\exp\left(-\left(\frac{u}{x}\right)^k\right)$$

Mean value and standard deviation $\mu_{X_T^{\text{max}}} = u\Gamma(1 - \frac{1}{k})$ $\sigma_{X_T^{\text{max}}}^2 = u^2 \left[\Gamma(1 - \frac{2}{k}) - \Gamma^2(1 - \frac{1}{k})\right]$

Extreme Type III – Weibull Min

When a probability density function is downwards limited at ${\mathcal E}$ and the lower tail falls off towards ${\mathcal E}$ in the form

$$F(x) = c(x - \mathcal{E})^k$$

then the maximum in the time interval *T* is said to be Type III extreme distributed

$$F_{X,T}^{\min}(x) = 1 - \exp\left(-\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^k\right)$$
$$f_{X,T}^{\min}(x) = \frac{k}{u-\varepsilon}\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^{k-1} \exp\left(-\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^k\right)$$

Mean value and standard deviation $\mu_{X_T^{\min}} = \varepsilon + (u - \varepsilon)\Gamma(1 + \frac{1}{k})$ $\sigma_{X_T^{\min}}^2 = (u - \varepsilon)^2 \left[\Gamma(1 + \frac{2}{k}) - \Gamma^2(1 + \frac{1}{k})\right]$

Return Period

The return period for extreme events T_R may be defined as:

$$T_{R} = n \cdot T = \frac{1}{(1 - F_{X,T}^{\max}(x))} T$$

Example:

Let us assume that - according to the cumulative probability distribution of the annual maximum traffic load - the annual probability that a truck load is larger than 100 ton is equal to 0.02 – then the return period of such heavy truck events is:

$$T_R = n \cdot T = \frac{1}{0.02} 1 = 50$$
 years

T=1 since we speak for annual probability of the extreme load event



Statistics and Probability Theory in

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Contents of Today's Lecture

- Presentation on the result of the classroom assessment
- Catching up with the lecture from last time
 - Continuous random processes
 - Extremes of random processes
- Overview of Estimation and Model Building
- **Probability Distribution Functions in Statistics**
- Estimators for Sample Descriptors Sample Statistics
 - statistical characteristics of the sample average
 - statistical characteristics of the sample variance
 - confidence intervals on estimators

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$$F_{X,nT}^{\max}(x) = \left(F_{X,T}^{\max}(x)\right)^n$$



Extreme Type I – Gumbel Max

When the upper tail of the probability density function falls off exponentially (exponential, Normal and Gamma distribution) then the maximum in the time interval *T* is said to be Type I extreme distributed

$$f_{X,T}^{\max}(x) = \alpha \exp(-\alpha (x-u) - \exp(-\alpha (x-u)))$$

$$F_{X,T}^{\max}(x) = \exp(-\exp(-\alpha(x-u)))$$

$$\mu_{X_T^{\max}} = u + \frac{\gamma}{\alpha} = u + \frac{0.577216}{\alpha}$$

$$\sigma_{X_T^{\max}} = \frac{\pi}{\alpha\sqrt{6}}$$

For increasing time intervals the variance is constant but the mean value increases as: $\sqrt{6}$

$$\mu_{X_{nT}^{\max}} = \mu_{X_{T}^{\max}} + \frac{\sqrt{6}}{\pi} \sigma_{X_{T}^{\max}} \ln(n)$$

Extreme Type II – Frechet Max

When a probability density function is downwards limited at zero and upwards falls off in the form

$$F_X(x) = 1 - \beta \left(\frac{1}{x}\right)^k$$

then the maximum in the time interval *T* is said to be Type II extreme distributed

$$F_{X,T}^{\max}(x) = \exp\left(-\left(\frac{u}{x}\right)^k\right)$$
$$f_{X,T}^{\max}(x) = \frac{k}{u}\left(\frac{u}{x}\right)^{k+1}\exp\left(-\left(\frac{u}{x}\right)^k\right)$$

Mean value and standard deviation $\mu_{X_T^{\text{max}}} = u\Gamma(1 - \frac{1}{k})$ $\sigma_{X_T^{\text{max}}}^2 = u^2 \left[\Gamma(1 - \frac{2}{k}) - \Gamma^2(1 - \frac{1}{k})\right]$

Extreme Type III – Weibull Min

When a probability density function is downwards limited at ε and the lower tail falls off towards ε in the form

$$F(x) = c(x - \mathcal{E})^k$$

then the minimum in the time interval *T* is said to be Type III extreme distributed

$$F_{X,T}^{\min}(x) = 1 - \exp\left(-\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^k\right)$$
$$f_{X,T}^{\min}(x) = \frac{k}{u-\varepsilon}\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^{k-1} \exp\left(-\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^k\right)$$

Mean value and standard deviation $\mu_{X_T^{\min}} = \varepsilon + (u - \varepsilon)\Gamma(1 + \frac{1}{k})$ $\sigma_{X_T^{\min}}^2 = (u - \varepsilon)^2 \left[\Gamma(1 + \frac{2}{k}) - \Gamma^2(1 + \frac{1}{k})\right]$

Return Period

The return period for extreme events T_R may be defined as:

$$T_R = n \cdot T = \frac{1}{(1 - F_{X,T}^{\max}(x))}$$

Example:

Let us assume that - according to the cumulative distribution function of the annual maximum traffic load the annual probability that a truck load larger than 100 ton is equal to 0.02 – then the return period of such heavy truck events is:

$$T_R = n \cdot T = \frac{1}{0.02} \Longrightarrow n = \frac{1}{1 \cdot 0.02} = 50$$
 years

Overview of Estimation and Model Building

How do engineers establish knowledge



Overview of Estimation and Model Building

Different types of information is used when developing engineering models

- subjective information
- frequentististic information



Overview of Estimation and Model Building

Model building may be seen to consist of five steps

- 1) Assessment and statistical quantification of the available data
- 2) Selection of distribution function
- 3) Estimation of distribution parameters
- 4) Model verification
- 5) Model updating

In the classical statistical theory a number of probability distribution functions which may all be derived from the normal distribution function are repeatedly used for assessment and testing purposes.

These derived probability distribution functions are the :

- > Chi-square distribution
- > Chi-distribution
- > t-distribution
- > F-distribution

The Chi-square (χ^2) distribution

When $X_i, i = 1, 2, ... n$

are standard Normal distributed and independent random variables then the sum of the squares of the random variables i.e.

$$Y_n = \sum_{i=1}^n X_i^2$$

is said to be Chi-square distributed

It is seen that the Chi square distribution is regenerative i.e. sums of Chi-square distributed random variables are also Chi-square distributed

The Chi-square (χ^2) distribution

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Consider the simplest case with n=1, i.e. : $Y_1 = X^2$

Then we can write

$$F_{Y_1}(y) = P(Y_1 \le y) = P(X^2 \le y) = P(-\sqrt{y} \le X \le +\sqrt{y})$$

= $F_X(\sqrt{y}) - F_X(-\sqrt{y}) = F_X(\sqrt{y}) - (1 - F_X(\sqrt{y})) =$
= $2F_X(\sqrt{y}) - 1$

and we get

$$f_{Y_1}(y) = \frac{dF_{Y_1}(y)}{dy} = \frac{d(2F_X(\sqrt{y}) - 1)}{dy} = y^{-\frac{1}{2}} f_X(\sqrt{y}) = \frac{1}{\sqrt{2\pi y}} \exp(-\frac{1}{2}y)$$

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The Chi-square probability density function is given as $f_{Y_n}(y_n) = \frac{y_n^{(n/2-1)}}{2^{n/2}\Gamma(n/2)} \exp(-y_n/2), \qquad y_n \ge 0$

The variance

$$\sigma_{Y_n}^2=2n$$

 $\Gamma(x) = \int_{0}^{\infty} e^{-t} t^{x-1} dt$ is the complete Gamma function

for large *n* the Chi-square distribution converges to a Normal distribution – Central Limit Theorem

The Chi-square probability density function

Chi-square probability density function



The Chi (χ) distribution

When a random variable Z is given as the square root of a Chi-square distributed random variable i.e.

$$Z = \sqrt{Y_n}$$

it is said to be Chi-distributed witn *n* degrees of freedom



The Chi (χ) distribution

Assume that Y_n is Chi-square distributed with n degrees of freedom

If
$$Z = \sqrt{Y_n}$$
 then we can write
 $F_Z(z) = P(Z \le z) = P(\sqrt{Y_n} \le z) = P(Y_n \le z^2) = F_{Y_n}(z^2)$

and we get

$$f_{Z}(z) = \frac{dF_{Z}(z)}{dz} = \frac{dF_{Y_{n}}(z^{2})}{dz} = 2zf_{Y_{n}}(z^{2}) = \frac{z^{n-1}}{2^{n/2-1}\Gamma(n/2)}\exp(-\frac{1}{2}z^{2})$$
The Chi probability density function is given as

$$f_{Z}(z) = \frac{z^{(n-1)}}{2^{n/2-1}\Gamma(n/2)} \exp(-z^{2}/2), \qquad z \ge 0$$

The mean value is
$$\mu_z = \sqrt{2} \frac{\Gamma((n+1)/2)}{\Gamma(n/2)}$$

The variance

$$\sigma_z^2 = n - 2 \frac{\Gamma^2((n+1)/2)}{\Gamma^2(n/2)}$$



The Chi probability density function

Chi probability density function



The (Student's) t distribution

When a random variable *s* is given as standard Normal distributed, devided by a Chi distributed random variable i.e.

$$S = \frac{X}{\sqrt{\sum_{i=1}^{n} X_i^2}} = \frac{X}{\frac{\sqrt{Y_n}}{n}} = \frac{X}{\frac{Z}{n}} = \frac{nX}{Z}$$

n

it is said to be *t*-distributed witn *n* degrees of freedom

For large *n* the *t*-distribution converges to a Normal distribution.

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 $-\infty < s < \infty$

The (Student's) *t* probability density function is given as

$$f_{s}(s) = \frac{\Gamma((n+1)/2)}{\sqrt{n\pi} \Gamma(n/2)} \left(1 + \frac{s^{2}}{n}\right)^{-(n+1)/2},$$

The mean value is zero

The variance
$$\sigma_{i}^{2}$$

$$\frac{1}{s}^2 = \frac{n}{n-2}$$



The (Student's) t probability density function

t-distribution



The F distribution

When a random variable Q is given as the ratio between two Chi-square distributed random variables i.e.

$$Q = \frac{Y_{n_1}}{Y_{n_2}}$$

it is said to be *F*-distributed with parameters $n_{1'}$ n_2



The F probability density function is given as

$$f_Q(q) = \frac{\Gamma((n_1 + n_2)/2)q^{(n_1 - 2)/2}(1 + q)^{-(n_1 + n_2)/2}}{\Gamma(n_1/2)\Gamma(n_2/2)}, \qquad q \ge 0$$

The mean value is
$$\mu_Q = \frac{n_2}{n_2 - 2}$$
, $n_2 > 2$

The variance

$$\sigma_Q^2 = \frac{2n_2^2(n_1 + n_2 - 2)}{n_1(n_2 - 2)^2(n_2 - 4)}, \qquad n_2 > 4$$

The F probability density function

F-distribution



Summary of derived probability density functions:

Distribution Type

When

- > Chi-square distribution
- > Chi-distribution
- > t-distribution
- > F-distribution

sum of squared N(0;1) square root of Chi-square ratio of N(0;1) to Chi/n ratio of two Chi-square



Example Chi distribution

In the field measurements have been performed of *a* and *b* with the purpose to assess *c*





Example Chi distribution



It is assumed that the measurements of *a* and *b* are performed with the same absolute error *e* which is assumed to N(0; σ_e) i.e. Normal distributed, unbiased and with standard deviation σ_e .

Determine the statistical characteristics of the error in *c* when this is assessed using the measurements of *a* and *b*.





Knowing that the error propagates according to

$$\mathcal{E}_c = \sqrt{\mathcal{E}_a^2 + \mathcal{E}_b^2}$$

we realize that

$$\frac{\mathcal{E}_{c}}{\sigma_{\varepsilon}} = \sqrt{\left(\frac{\mathcal{E}_{a}}{\sigma_{\varepsilon}}\right)^{2} + \left(\frac{\mathcal{E}_{b}}{\sigma_{\varepsilon}}\right)^{2}}$$

Example Chi distribution

is Chi distributed with 2 degrees of freedom



Probability Distribution Functions in Statistics a Example Chi distribution The probability density function of $Z = \frac{\mathcal{E}_c}{\sigma_{\mathcal{E}}}^b$ can thus be determined from

$$f_Z(z) = z \exp(-0.5z^2), \qquad z \ge 0$$

yielding
$$f_{\varepsilon_c}(\varepsilon_c) = \frac{\varepsilon_c}{\sigma_{\varepsilon}} \exp(-0.5\varepsilon_c^2/\sigma_{\varepsilon}^2), \quad \varepsilon_c \ge 0$$

where it was used that for y = g(x) we have $f_y(y) = \left| \frac{dg^{-1}}{dv} \right| f_X(g^{-1}(y))$

The first step when new data are achieved is to assess the data



We want to have a look at the statistical characteristics of such sample statistics – in order to better understand the information they contain

Assume we have a yet unknown sample of experiment outcomes

 $X_i, i = 1, 2, ... n$

generated by the cumulative distribution functions

$$F_{X_i}(x_i, \mathbf{p}) = F_X(x, \mathbf{p}), i = 1, 2, ... n$$

then we can write the sample statistics for the

sample mean

$$\overline{X} = \frac{1}{n} \sum_{i=1}^{n} X_i$$

sample variance

$$S^{2} = \frac{1}{n} \sum_{i=1}^{n} (X_{i} - \overline{X})^{2}$$

The sample statistics are random variables because the experiment outcomes have not yet been realized – however we can evaluate the expected value and the variance of the sample statistics, i.e. for the sample mean we get :

$$E[\overline{X}] = E\left[\frac{1}{n}\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n}\sum_{i=1}^{n}E[X_{i}] = \frac{1}{n}n \cdot \mu_{X} = \mu_{X}$$

$$Var[\overline{X}] = Var\left[\frac{1}{n}\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n^{2}}Var\left[\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n^{2}}\sum_{i=1}^{n}Var[X_{i}]$$
$$= \frac{1}{n^{2}}\sum_{i=1}^{n}E[(X_{i} - \mu_{X})^{2}] = \frac{1}{n}\sigma_{X}^{2}$$

The probability density function for the sample average can be assumed to be a Normal distribution – Central Limit Theorem



For the sample variance we get:

$$E\left[S^{2}\right] = E\left[\frac{1}{n}\sum_{i=1}^{n}\left(X_{i}-\overline{X}\right)^{2}\right] = \frac{1}{n}E\left[\sum_{i=1}^{n}\left(\left(X_{i}-\mu_{\mathbf{X}}\right)-\left(\overline{X}-\mu_{\mathbf{X}}\right)\right)^{2}\right]$$
$$= \frac{1}{n}\left(\sum_{i=1}^{n}E\left[\left(X_{i}-\mu_{\mathbf{X}}\right)^{2}\right]-nE\left[\left(\overline{X}-\mu_{\mathbf{X}}\right)^{2}\right]\right)$$
$$= \frac{1}{n}\left(nE\left[\left(X_{i}-\mu_{\mathbf{X}}\right)^{2}\right]-nE\left[\left(\overline{X}-\mu_{\mathbf{X}}\right)^{2}\right]\right) =$$
$$= \frac{1}{n}\left(n\sigma_{x}^{2}-n\frac{\sigma_{x}^{2}}{n}\right) = \sigma_{x}^{2}-\frac{1}{n}\sigma_{x}^{2} = \frac{(n-1)}{n}\sigma_{x}^{2}$$

The expected value of the sample variance is thus different from the variance – biased !

We can however easily identify an unbiased estimator for the variance as:

$$S_{unbiased}^{2} = \frac{n}{n-1}S^{2} = \frac{1}{n-1}\sum_{i=1}^{n}(X_{i} - \overline{X})^{2}$$

Not *n* as in the sample variance



The goodness of an estimator cannot be judged upon whether it is biased or not alone – other properties are important such as

- efficiency
- invariance
- consistent
- sufficiency
- robustness

- least mean square error E[(s²-s²)] $h(\overline{\theta}) = \overline{h(\theta)}$
- converge to the true values make maximum use of the data sensitivity to omission of individual data

we will not consider these in detail – just remember that these considerations may also be important

In the previous we have seen that estimators of e.g. the mean value are associated with uncertainty and we have established expressions to determine their mean value and variance –

Based on this information we are also able to determine so called confidence intervals on the estimators.

For the case where it is assumed that the variance is known and only the mean value is uncertain the socalled <u>double sided and symmetrical confidence</u> <u>interval on the mean value</u> is given by

$$P\left[-k_{\alpha/2} < \frac{\overline{X} - \mu_X}{\sigma_X \frac{1}{\sqrt{n}}} < k_{\alpha/2}\right] = P\left[-k_{\alpha/2} \sigma_X \frac{1}{\sqrt{n}} < \overline{X} - \mu_X < k_{\alpha/2} \sigma_X \frac{1}{\sqrt{n}}\right] = 1 - \alpha$$

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In words the confidence interval defines an interval within which e.g. the true mean value will lie with a probability $1-\alpha$

$$P\left[-k_{\alpha/2}\sigma_{X}\frac{1}{\sqrt{n}} < \overline{X} - \mu_{X} < k_{\alpha/2}\sigma_{X}\frac{1}{\sqrt{n}}\right] = 1 - \alpha$$

For the case where α = 0.05, *n* = 16 and σ_X = 20 we get

$$k_{\alpha/2} = \Phi^{-1}\left(1 - \frac{\alpha}{2}\right) = \Phi^{-1}\left(1 - \frac{0.05}{2}\right) = 1.96$$

 $P[-9.8 < \overline{X} - \mu_X < 9.8] = 0.95$

If we then observe that the sample mean is equal to e.g. 400 we know that with a probability equal to 0.95 the true mean will lie within the interval

$$P\left[-9.8 < \bar{X} - \mu_X < 9.8\right] = 0.95$$

and so: $390.2 < \mu_X < 409.8$

Typically confidence intervals are considered for mean values, variances and characteristic values – e.g. lower percentile values.

Confidence intervals represent/describe the (statistical) uncertainty due to lack of data.

The number of available data has a significant importance for the confidence interval - using the same example as in the previous the confidence interval depends on *n* as shown below



Statistics and Probability Theory in

Civil, Surveying and Environmental Engineering

Prof. Dr. Michael Havbro Faber Swiss Federal Institute of Technology ETH Zurich, Switzerland



Contents of Todays Lecture

- Overview of Estimation and Model Building
- A short Summary of the Previous Lecture
- Estimators for Sample Descriptors
- Testing for Statistical Significance
 - The hypothesis testing procedure
 - Testing of the mean with known variance
 - Testing of the mean with unknown variance
 - Testing of the variance
 - Test of two or more data sets

Overview of Estimation and Model Building

Different types of information is used when developing engineering models

- subjective information
- frequentistic information





Continuous random processes

A continuous random process is a random process which has realizations continuously over time and for which the realizations belong to a continuous sample space.



Realization of continuous scalar valued random process

If the extremes within the period *T* of an ergodic random process *X*(*t*) are independent and follow the distribution:

$$F_{X,T}^{\max}(x) = P(\max_{T} X \le x)$$

then the extremes of the same process within the period:

 $n \cdot T$ will follow the distribution:

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$$F_{X,nT}^{\max}(x) = P\left(\left\{\max_{T_1} X \le x\right\} \bigcap \left\{\max_{T_2} X \le x\right\} \dots \bigcap \left\{\max_{T_n} X \le x\right\}\right)$$
$$= P\left(\bigcap_{i=1}^n \left\{\max_{T_i} X \le x\right\}\right)$$
$$= \prod_{i=1}^n P\left(\max_{T_i} X \le x\right)$$
$$= \left(F_{X,T}^{\max}(x)\right)^n$$

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If the extremes within the period *T* of an ergodic random process *X*(*t*) are independent and follow the distribution:

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$$= P\left(\bigcap_{i=1}^n \left\{\max_{T_i} X \le x\right\}\right)$$

$$= \prod_{i=1}^n P\left(\max_{T_i} X \le x\right)$$

$$= \left(F_{X,T}^{\max}(x)\right)^n$$
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Based on independent Normal distributed random variables we could derive the following distributions:

Distribution Type

- > Chi-square distribution
- > Chi-distribution
- > *t*-distribution
- > F-distribution

When

sum of squared N(0;1) square root of Chi-square ratio of N(0;1) to Chi/*n* ratio of two Chi-square

Example Chi distribution

In the field, measurements have been performed of *a* and *b* with the purpose to assess *c*





Example Chi distribution



It is assumed that the measurements of a and b are performed with the same absolute error ε which is assumed to N(0; σ_{ε}) i.e. Normal distributed, unbiased and with standard deviation σ_{ε} .

Determine the statistical characteristics of the error in *c* when this is assessed using the measurements of *a* and *b*.





Knowing that the error propagates according to

$$\mathcal{E}_c = \sqrt{\mathcal{E}_a^2 + \mathcal{E}_b^2}$$

we realize that

$$\frac{\mathcal{E}_{c}}{\sigma_{\varepsilon}} = \sqrt{\left(\frac{\mathcal{E}_{a}}{\sigma_{\varepsilon}}\right)^{2} + \left(\frac{\mathcal{E}_{b}}{\sigma_{\varepsilon}}\right)^{2}}$$

Example Chi distribution

is Chi distributed with 2 degrees of freedom

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$$f_Z(z) = z \exp(-0.5z^2), \qquad z \ge 0$$

yielding
$$f_{\varepsilon_c}(\varepsilon_c) = \frac{\varepsilon_c}{\sigma_{\varepsilon}} \exp(-0.5\varepsilon_c^2/\sigma_{\varepsilon}^2), \quad \varepsilon_c \ge 0$$

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We want to have a look at the statistical characteristics of such sample statistics – in order to better understand the information they contain

Assume we have a yet unknown sample of experiment

outcomes
$$X_i$$
, $i = 1, 2, ... n$

generated by the cumulative distribution functions

$$F_{X_i}(x_i, \mathbf{p}) = F_X(x, \mathbf{p}), i = 1, 2, ... n$$

then we can write the sample statistics for the

sample mean

sample variance

$$S^{2} = \frac{1}{n} \sum_{i=1}^{n} (X_{i} - \overline{X})^{2}$$

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 $\overline{X} = \frac{1}{n} \sum_{i=1}^{n} X_i$

The sample statistics are random variables,

because the experiment outcomes have not yet been realized –

however we can evaluate the expected value and the variance of the sample statistics, i.e. for the sample mean we get :

$$E[\overline{X}] = E\left[\frac{1}{n}\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n}\sum_{i=1}^{n}E[X_{i}] = \frac{1}{n}n \cdot \mu_{X} = \mu_{X}$$

$$Var\left[\overline{X}\right] = Var\left[\frac{1}{n}\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n^{2}}Var\left[\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n^{2}}\sum_{i=1}^{n}Var\left[X_{i}\right] = \frac{1}{n}\sigma_{X}^{2}$$

The probability density function for the sample average can be assumed to be a Normal distribution – Central Limit Theorem



For the sample variance we get:

$$E\left[S^{2}\right] = E\left[\frac{1}{n}\sum_{i=1}^{n}(X_{i}-\overline{X})^{2}\right] = \frac{1}{n}E\left[\sum_{i=1}^{n}((X_{i}-\mu_{\mathbf{X}})-(\overline{X}-\mu_{\mathbf{X}}))^{2}\right]$$
$$= \frac{1}{n}\left[\sum_{i=1}^{n}E\left[(X_{i}-\mu_{\mathbf{X}})^{2}\right] - nE\left[(\overline{X}-\mu_{\mathbf{X}})^{2}\right]\right]$$
$$= \frac{1}{n}\left(n \cdot E\left[(X_{i}-\mu_{\mathbf{X}})^{2}\right] - nE\left[(\overline{X}-\mu_{\mathbf{X}})^{2}\right]\right) =$$
$$= \frac{1}{n}\left(n \cdot \sigma_{X}^{2} - n\frac{\sigma_{X}^{2}}{n}\right)$$
$$= \sigma_{X}^{2} - \frac{1}{n}\sigma_{X}^{2} = \frac{(n-1)}{n}\sigma_{X}^{2}$$
The expected value of the sample variance is thus different from the variance - biased 1

We can however easily identify an unbiased estimator for the variance as:





- In the previous we have seen that estimators of e.g. the mean value are associated with uncertainty and we have established expressions to determine their mean value and variance.
- Based on this information we are also able to determine so called confidence intervals on the estimators.
- Confidence intervals may be understood as intervals within which e.g. the mean value can be found
- Confidence is expressed in terms of probability



We may e.g. establish a confidence interval for the mean value.

For the case where it is assumed that the mean value is uncertain and the variance is known the so-called double sided and symmetrical confidence interval on the mean value is given by



In words: the confidence interval defines an interval within which the sample average will be located with a probability $1-\alpha$

$$P\left[-k_{\alpha/2}\sigma_{X}\frac{1}{\sqrt{n}}<\overline{X}-\mu_{X}< k_{\alpha/2}\sigma_{X}\frac{1}{\sqrt{n}}\right]=1-\alpha$$

Known std. dev. / True mean Sample size Sample average

The confidence interval may be determined using the assumption that the mean value is Normal distributed whereby there is:

$$k_{\alpha/2} = \Phi^{-1}\left(1 - \frac{\alpha}{2}\right) = \Phi^{-1}\left(1 - \frac{0.05}{2}\right) = 1.96$$

For the case where α = 0.05, *n* = 16 and σ_X = 20 we get

$$P\left[-1.96 < \frac{\bar{X} - \mu_X}{20\frac{1}{\sqrt{n}}} < 1.96\right] = 1 - 0.05$$

$$P\left[-9.8 < \overline{X} - \mu_X < 9.8\right] = 0.95$$



 If we then observe that the sample mean is equal to e.g. 400 we know that with a probability equal to 0.95 the true mean will lie within the interval

$$P\left[-9.8 < \bar{X} - \mu_X < 9.8\right] = 0.95$$

and so:
$$390.2 < \mu_X < 409.8$$

- Typically confidence intervals are considered for mean values, variances and characteristic values – e.g. lower percentile values.
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The number of available data has a significant importance for the confidence interval - using the same example as in the previous the confidence interval depends on *n* as shown below



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- Selection of Distribution Function
 - Model selection by use of probability paper

In the previous lecture we looked at:

Estimators for Sample Descriptors

Confidence Intervals on Estimators



Sample descriptors are simply e.g.

The sample mean value

The sample variance

What did we learn?

The sample descriptors are associated with uncertainty due to statistical uncertainty (epistemical uncertainty)



The sample mean value is an unbiased descriptor



$$E[\overline{X}] = E\left[\frac{1}{n}\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n}\sum_{i=1}^{n}E[X_{i}] = \frac{1}{n}n \cdot \mu_{X} = \mu_{X}$$

$$Var\left[\bar{X}\right] = Var\left[\frac{1}{n}\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n^{2}}Var\left[\sum_{i=1}^{n}X_{i}\right] = \frac{1}{n^{2}}\sum_{i=1}^{n}Var\left[X_{i}\right] = \frac{1}{n}\sigma_{X}^{2}$$

The sample variance is biased !

$$E\left[S^{2}\right] = E\left[\frac{1}{n}\sum_{i=1}^{n}(X_{i}-\bar{X})^{2}\right] = \frac{1}{n}E\left[\sum_{i=1}^{n}((X_{i}-\mu_{\bar{X}})-(\bar{X}-\mu_{\bar{X}}))^{2}\right]$$
$$= \frac{1}{n}\left(\sum_{i=1}^{n}E\left[(X_{i}-\mu_{\bar{X}})^{2}\right]-nE\left[(\bar{X}-\mu_{\bar{X}})^{2}\right]\right)$$
$$= \frac{1}{n}\left(nE\left[(X_{i}-\mu_{\bar{X}})^{2}\right]-nE\left[(\bar{X}-\mu_{\bar{X}})^{2}\right]\right)$$
$$= \frac{1}{n}\left(n\sigma_{\bar{X}}^{2}-n\frac{\sigma_{\bar{X}}^{2}}{n}\right) = \sigma_{\bar{X}}^{2}-\frac{1}{n}\sigma_{\bar{X}}^{2} = \frac{(n-1)}{n}\sigma_{\bar{X}}^{2}$$
$$S_{unbiased}^{2} = \frac{n}{n-1}S^{2}$$
$$= \frac{n}{n-1}\left(\frac{1}{n}\sum_{i=1}^{n}(X_{i}-\bar{X})^{2}\right)$$
$$= \frac{1}{n-1}\sum_{i=1}^{n}(X_{i}-\bar{X})^{2}$$

• Due to the uncertainty associated with the descriptors (e.g. sample mean) we dont know their exact value

 We can however determine intervals where we can find them with a given probability

These intervals we call confidence intervals!



The number of available data has a significant importance for the confidence interval - using the same example as in the previous the confidence interval depends on *n* as shown below



Overview of Estimation and Model Building

Different types of information is used when developing engineering models

- subjektive information
- frequentististic information



Engineering dilemma :

Draw simple conclusions based on limited data with a high degree of variability –

E.g. : Make a few "on site" tests to verify a calculation model of the soil strength characteristics

Use observations of traffic crossing a bridge to check if design traffic volume assumptions are valid

Collect ground water "samples" to verify that the water is of drinking quality



It is important that such conclusions are drawn on a basis which is consistent and transparent – i.e. the conclusions should reflect the evidence (data) and a given formalism in regard to what evidence triggers which conclusions

One highly utilized and useful formalism for supporting such conclusions is to

- **1** Formulate hypothesis
- 2 Test hypothesis

We shall have a look into this approach is some detail in the following



- 1 The first step is to formulate a null-hypothesis H_0 e.g. postulating that a sample statistic (e.g. sample mean) is equal to a given value
- 2 The next step is to formulate an operating rule on the basis of which the null-hypothesis can either be accepted or rejected – given the evidence (test results) – such an operating rule is often defined by an interval D within which the observed sample statistic has to be in – for the null-hypothesis to be accepted - rejecting the nullhypothesis H₀ corresponds to accepting the alternate H₁ hypothesis
- 3 Select a significance level a for conducting the test where a is the probability that the hypothesis will be rejected even though it is true (Type I error) – in this way a also influences the probability that the null-hypothesis is accepted even though it is false (Type II error)

- 4 Calculate the value of D corresponding to a calculate also if relevant the probability of performing a Type II error
- 5 Perform the planned tests and evaluate the observed sample statistic – check if the null-hypothesis should be rejected or accepted
- 6 Given that the null-hypothesis is not supported by the evidence (data) the null-hypothesis is rejected at significance level a otherwise it is accepted.

The hypothesis testing procedure may be visualized as follows



Typical Tests in Engineering

- Testing of the mean with known variance
- Testing of the mean with unknown variance
- Testing of the variance
- Test of two or more data sets



Example – chloride induced corrosion of concrete structures





Consider an example where we want to verify whether the chloride concentration on the surface of a concrete structure is in compliance with our design assumptions





we assume that we know the std. dev. of the surface chloride concentration – equal to 0.04%

The operating rule is formulated as: Accept the Null-hypothesis at the α -level if

 $0.3 - \Delta \le \overline{X} \le 0.3 + \Delta$

Testing of the mean – with known variance

The acceptance criteria may be determined for given α by

$$P(0.3 - \Delta \le \overline{X} \le 0.3 + \Delta) = 1 - \alpha$$

Choosing $\alpha = 0.1$, n= 10 experiments and assuming that the sample average is normal distributed we get





Testing of the mean – with known variance

If the sample average lies in the interval the Null-hypothesis H₀ should be accepted

 $\left[0.28 \le \bar{x} \le 0.32\right]$

Assume that 10 experiments are carried out and the following results are obtained

 $\mathbf{x} = (0.33, 0.32, 0.25, 0.31, 0.28, 0.27, 0.29, 0.3, 0.27, 0.28)^T$

with sample average $\mu = 0.29$ - it is concluded that the Null-hypothesis should be accepted at the 0.1 level.

Testing of the mean – with unknown variance

If now it is assumed that the variance is unknown the following sample statistic must be considered

$$T = \frac{X - \mu}{\frac{S_{unbiased}}{\sqrt{n}}}$$

which may be realized to be t-distributed with *n*-1 degree of freedom

The operating rule is then: accept H_0 if $-\Delta \le T \le \Delta$

The critical value can be calculated from: $P(-\Delta \le T \le \Delta) = 1 - \alpha$

from which $\Delta = 1.83$ is determined using the *t*-distribution with 9 degrees of freedom

Testing of the mean – with unknown variance

Assuming the same experiment outcomes as before we get the same sample average but now the variance is given by

$$s_{unbiased} = \sqrt{\frac{1}{(n-1)} \sum_{i=1}^{n} (x_i - \bar{x})^2} = 0.025$$

and the *t*-statistic becomes

$$t = \frac{(0.29 - 0.3)\sqrt{10}}{0.025} = -1.27$$

which is within the interval given by $\pm \Delta$ (= \pm 1.83)

Thus the Null-hypothesis should not be rejected

Testing of the variance

Consider as an example the case where the variance of the fatigue lifes of welded joints is attempted reduced by means of weld surface treatment.





As experiments are very expensive only a few data are available to verify the effect of the weld surface treatment.



Testing of the variance

We may as Null-hypothesis postulate that the variance of the fatigue lifes with the surface treatment is smaller that the variance before the surface treatment i.e. :

$$\sigma_{new}^2 \leq \sigma_{old}^2$$

The operating rule is then to accept the Null hypothesis if

 $S^2 \leq \Delta$

where Δ is determined from $P[S^2 \leq \Delta] = 1 - \alpha$

and it is used that S² is Chi-square distributed with *n* degrees of freedom



Testing of more than one data set

Typically we are in a situation where we have two or more data sets each not very large – and we would like to know how the data compare in terms of :

- mean values
 variances
 Test for equal mean values
 Test for equal variances
- correlation Test for zero correlation



Testing for equal mean values

Here we assume that we have two data sets

$$\mathbf{x} = (x_1, x_2, ..., x_k)^T$$
 $\mathbf{y} = (y_1, y_2, ..., y_l)^T$

being realizations of the random variables X and Y both assumed to be normal distributed with mean values $\mu_{X'}$, μ_{Y} and variances $\sigma_{X'}$, σ_{Y}

the statistic $T = \overline{X} - \overline{Y}$

is Normal distributed with mean value $\mu_{\bar{X}-\bar{Y}} = \mu_X - \mu_Y$

and variance

$$\sigma_{\overline{X}-\overline{Y}}^2 = \frac{\sigma_X^2}{k} + \frac{\sigma_Y^2}{l}$$
Testing for Statistical Significance

Testing for equal mean values

For α equal to 0.1 Δ can be calculated as

$$P(\overline{X} - \overline{Y} \le \Delta) = 1 - \alpha \qquad \Rightarrow \quad \Delta = 1.28 \sqrt{\frac{\sigma_X^2}{k} + \frac{\sigma_Y^2}{l}}$$



Testing for Statistical Significance

Testing for equal variances

A test for equal variances can be performed by considering the following statistic

$$T = \frac{S_{X,unbiased}^2}{S_{Y,unbiased}^2}$$

which is seen to be the ratio between two Chi-square distributed random variables – and *T* is thus *F*-distributed with parameters *k* and *I*.

The Null-hypothesis H_o would be that

$$\sigma_X^2 = \sigma_Y^2$$

and the operating rule to accept H_0 if

where Δ is determined from

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 $T \leq \Delta$ $P(T \leq \Delta) = 1 - \alpha$

Testing for Statistical Significance

Some considerations regarding testing for significance

Test for statistical significance can be formulated for a variety of different types of problems

we must be very careful not to "over estimate" the value of the significnace tests because the hypothesis can be formulated in different ways and using different significance levels a consequently it is in principle possible to prove anything –

the different choises have direct effect on the probability of performing Type I and Type II errors – which may be related to significant economical consequences

the formulation of hypothesis and the choise of significance levels should be treated as a decision problem - which will be treated later.

Overview of Estimation and Model Building

Different types of information is used when developing engineering models

- subjektive information
- frequentististic information





Selection of probability distribution function

In general the distribution function for a given random variable or random process must be chosen on the basis of

Frequentistic information: DataPhysical arguments:Engineering understanding

A formalized classical approach is to

- 1 postulate a hypothesis for the probability distribution family
- 2 estimate the parameters of the postulated probability distribution
- 3 Perform a statistical test to reject/verify the hypothesis

Selection of probability distribution function

In engineering application it is often the case that

the available data is too sparse

to be able to support/reject the hypothesis of a given probability distribution – with a reasonable significance

Therefore it is necessary to use common sence i.e. :

First to consider physical reasons for selecting a given distribution

Thereafter to check if the available data are in gross contradiction with the selected distribution



Model selection by use of probability paper

Probability paper is constructed such that when a given probability distribution is plotted on the paper it will have the shape of a straight line



Model selection by use of probability paper

Example – probability paper for the normal probability distribution function

$$F_X(x) = \Phi(\frac{x - \mu_X}{\sigma_X})$$
$$x = \Phi^{-1}(F_X(x)) \cdot \sigma_X + \mu_X$$

The y-axis scale is non-linear



Model selection by use of probability paper – graphical approach



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Model selection by use of probability paper

The sample probability distribution function may be established from the ordered sample as

$$F_X(x_i) = \frac{i}{N+1}$$

Example – concrete compression strength

Normal probability paper



i	X _i	F _x (x _i)	$\Phi^{-1}(F(x_i))$
1	24.4	0.047619	-1.668391
2	27.6	0.095238	-1.309172
3	27.8	0.142857	-1.067571
4	27.9	0.190476	-0.876143
5	28.5	0.238095	-0.712443
6	30.1	0.285714	-0.565949
7	30.3	0.333333	-0.430727
8	31.7	0.380952	-0.302981
9	32.2	0.428571	-0.180012
10	32.8	0.47619	-0.059717
11	33.3	0.52381	0.059717
12	33.5	0.571429	0.180012
13	34.1	0.619048	0.302981
14	34.6	0.666667	0.430727
15	35.8	0.714286	0.565949
16	35.9	0.761905	0.712443
17	36.8	0.809524	0.876143
18	37.1	0.857143	1.067571
19	39.2	0.904762	1.309172
20	39.7	0.952381	1.668391

Model selection by use of probability paper

Plotting the sample probability distribution function in the probability paper yields



Statistics and Probability Theory in

Civil, Surveying and Environmental Engineering

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Contents of Todays Lecture

- The Results of the Assessment of the Lecture
- Short Summary of the Previous Lecture
- Overview of Estimation and Model Building
- Estimation of Distribution Parameters
 - The method of moments
 - The method of maximum likelihood



In the previous lecture we introduced the concept of

hypothesis testing

- testing of the mean
- testing of the variance
- testing of more data sets

and we also introduced the concept of

probability paper

- supporting the choice of a given probabilistic model based on data/observations



• hypothesis testing – which are the steps!



The design assumption: The mean surface chloride concentration is 0.3%

Knowledge: Standard deviation of the surface chloride concentration – equal to 0.04%



Hypothesis (Ho hypothesis): Design assumption is correct!

Operating rule/testing approach

Given that we know the standard deviation we know that the uncertain mean is normal distributed – we thus have a normal distributed test statistic *T*

 $0.3 - \Delta \leq T \leq 0.3 + \Delta$



The test acceptance criteria: The operating rule must be fulfilled with a probability of $1 - \alpha$.

$$P(0.3 - \Delta \le T \le 0.3 + \Delta) = 1 - \alpha$$

Assessing acceptance criteria: The interval for the operating rule is

determined as:





 $\Phi\left(\frac{x_U - \mu}{\sigma}\right) - \Phi\left(\frac{x_L - \mu}{\sigma}\right) = \Phi\left(\frac{(0.3 + \Delta) - 0.3}{\frac{0.04}{\sqrt{10}}}\right) - \Phi\left(\frac{(0.3 - \Delta) - 0.3}{\frac{0.04}{\sqrt{10}}}\right) = 0.9 \qquad \Rightarrow \qquad \Delta = 0.0208 \qquad [0.28 \le t \le 0.32]$

Perform test and check for acceptance Collect samples and calculte the mean value

 $\mathbf{x} = (0.33, 0.32, 0.25, 0.31, 0.28, 0.27, 0.29, 0.3, 0.27, 0.28)^T \Longrightarrow t = 0.29$

Conclusion

The validity of design assumptions cannot be rejected at the 0.1 significance level

• Probability paper – what is the idea!

Fundamentally what we want to do is to check whether data/observations follow a given cumulative distribution function

If they do we have support for assuming that the uncertain phenomenon which generated the data can be modelled by the given cumulative distribution function

The concept of probability paper provides us a standardized manner to perform this check



Probability paper – what is the idea!

We construct probability paper for a given family of cumulative distribution functions such that a plot of the cumulative distribution follows a straight line in the paper

In order to do that we perform an non-linear transformation of the y-axis of the usual CDF plot



Probability paper – what is the idea!

When we have the paper (we can construct it our selves or buy it in the book store ⁽ⁱ⁾ we can plot observed values as a quantile-plot into the paper

i	X _i	F _x (x _i)	$\Phi^{-1}(F(x_i))$
1	24.4	0.047619	-1.668391
2	27.6	0.095238	-1.309172
3	27.8	0.142857	-1.067571
4	27.9	0.190476	-0.876143
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6	30.1	0.285714	-0.565949
7	30.3	0.333333	-0.430727
8	31.7	0.380952	-0.302981
9	32.2	0.428571	-0.180012
10	32.8	0.47619	-0.059717
11	33.3	0.52381	0.059717
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13	34.1	0.619048	0.302981
14	34.6	0.666667	0.430727
15	35.8	0.714286	0.565949
16	35.9	0.761905	0.712443
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18	37.1	0.857143	1.067571
19	39.2	0.904762	1.309172
20	39.7	0.952381	1.668391

$$F_X(x_i) = \frac{i}{N+1}$$



If the *q*-plot is close to straight in the important regions we have support for our model!

Overview of Estimation and Model Building

Different types of information is used when developing engineering models

- subjective information
- frequentististic information



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We assume that we have identified a plausible family of probability distribution functions – as an example :

Normal Distribution

Weibull distribution

$$f_X(x) = \frac{1}{\sigma\sqrt{2\pi}} exp\left(-\frac{1}{2}\left(\frac{x-\mu}{0}\right)^2\right) \qquad f_X(x) = \frac{k}{\mu-\varepsilon} \left(\frac{x-\varepsilon}{u-\varepsilon}\right)^{k-1} exp\left(-\left(\frac{x-\varepsilon}{u-\varepsilon}\right)^k\right)$$

and thus now need to determine – estimate - its parameters

 $\boldsymbol{\theta} = (\boldsymbol{\theta}_1, \boldsymbol{\theta}_2, ..., \boldsymbol{\theta}_k)^T$

There are several methods for estimating the parameters of probability distribution functions, hereunder the so-called

- Point estimators
- Interval estimators

however, in the following we shall restrict ourselves to consider the

Method of moments

Method of maximum likelihood



The method of moments (MoM)

To start with we assume that we have data on the basis of which we can estimate the distribution parameters $\hat{\mathbf{x}} = (\hat{x}_1, \hat{x}_2, ..., \hat{x}_n)^T$

The idea behind the method of moments is to determine the distribution parameters such that the sample moments (from the data) and the analytical moments (from the assumed distribution) are identical.

$$m_j = \frac{1}{n} \sum_{i=1}^n x_i^j$$

$$\lambda_j = \lambda_j(\theta_1, \theta_2, ..., \theta_k) = \int_{-\infty}^{\infty} x^j \cdot f_X(x|\mathbf{\theta}) dx$$

Sample moments

Analytical moments

The method of moments (MoM)

If we assume that the considered probability distribution function has *n* parameters that we must estimate we thus need *n* equations, i.e.

$$m_{j} = \lambda_{j}(\boldsymbol{\theta}), j = 1, 2, ..., n$$

$$\bigcup$$

$$\frac{1}{n} \sum_{i=1}^{n} x_{i}^{j} = \int_{-\infty}^{\infty} x^{j} \cdot f_{X}(x|\boldsymbol{\theta}) dx, j = 1, 2, ..., n$$

Sample moment

Analytical moment



The method of moments (MoM)

Consider as an example the data regarding the concrete compressive strength –

Again we assume that the concrete compressive strength is normal distributed – "the normal distribution family"

The normal distribution family has two parameters – we need thus to establish two equations $n = \frac{1}{2} \frac{n}{2}$

$$m_{1} = \frac{1}{n} \sum_{i=1}^{n} \hat{x}_{i} \qquad \lambda_{1} = \int_{-\infty}^{\infty} x \cdot f_{X}(x|\mu,\sigma) dx$$
$$m_{2} = \frac{1}{n} \sum_{i=1}^{n} \hat{x}_{i}^{2} \qquad \lambda_{2} = \int_{-\infty}^{\infty} x^{2} \cdot f_{X}(x|\mu,\sigma) dx$$

The method of moments (MoM)

The sample moments are easily calculated as

$$m_1 = \frac{1}{20} \sum_{i=1}^n \hat{x}_i = 32.67 \qquad \qquad m_2 = \frac{1}{20} \sum_{i=1}^n \hat{x}_i^2 = 1083.36$$

The analytical moments can be established as function of the parameters

$$\lambda_{1} = \int_{-\infty}^{\infty} x \cdot \frac{1}{\sigma\sqrt{2\pi}} \exp(-0.5\frac{(x-\mu)^{2}}{\sigma^{2}}) dx \qquad \lambda_{2} = \int_{-\infty}^{\infty} x^{2} \cdot \frac{1}{\sigma\sqrt{2\pi}} \exp(-0.5\frac{(x-\mu)^{2}}{\sigma^{2}}) dx$$



The method of moments (MoM)

By formulating the following object function

 $g(\mu,\sigma) = (\lambda_1(\mu,\sigma) - m_1)^2 + (\lambda_2(\mu,\sigma) - m_2)^2$

The parameters estimation problem can be solved numerically using Excel Solver finding the parameters minimizing the object function

Let's have a look !



The Maximum Likelihood Method (MLM)

The idea behind the method of maximum likelihood is that

The parameters are determined such that the likelihood of the observations is maximized

The likelihood can be understood as the probability of occurrence of the observed data conditional on the model

The Maximum Likelihood Method may seem more complicated that the MoM but has a number of attractive properties which we shall see later



The Maximum Likelihood Method (MLM)

Let us assume that we know that outcomes of experiments are generated according to the normal distribution, i.e.:

$$f_X(x) = \frac{1}{\sigma\sqrt{2\pi}} exp\left(-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^2\right)$$

Then the likelihood *L* of one experiment outcome \hat{x} is calculated as:

$$L = \frac{1}{\sigma\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{\hat{x}-\mu}{\sigma}\right)^2\right)$$

The Maximum Likelihood Method (MLM)

Let us assume that we know that outcomes of experiments are generated according to the normal distribution, i.e.:

$$f_X(x) = \frac{1}{\sigma\sqrt{2\pi}} exp\left(-\frac{1}{2}\left(\frac{x-\mu}{\sigma}\right)^2\right)$$

If we have n experiment outcomes $\mathbf{k} = (x_1, \mathbf{k}, ..., x_n)^T$ the likelihood *L* becomes:

$$L(\mathbf{\theta}|\hat{\mathbf{x}}) = \prod_{i=1}^{n} \frac{1}{\sigma\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{\hat{x}_{i}-\mu}{\sigma}\right)^{2}\right)$$



The Maximum Likelihood Method (MLM)

The parameters θ are estimated as those maximizing the likelihood function or equivalently minimizes the – likelihood function i.e.:

$$\min_{\boldsymbol{\theta}} \left(-L(\boldsymbol{\theta} \big| \hat{\mathbf{x}}) \right)$$

It is avantageous to consider the log-likelihood function $l(\theta | \hat{x})$:

$$l(\mathbf{\theta}|\mathbf{x}) = \sum_{i=1}^{n} \log(f_X(\hat{x}_i|\mathbf{\theta}))$$

The Maximum Likelihood Method (MLM)

If the parameters θ are estimated as those minimizing the – log likelihood function i.e.: $\min_{\theta} (-l(\theta | \hat{\mathbf{x}}))$

It can be shown that the estimated parameters are normal distributed with

mean values $\boldsymbol{\mu}_{\Theta} = (\boldsymbol{\theta}_{1}^{*}, \boldsymbol{\theta}_{2}^{*}, ..., \boldsymbol{\theta}_{n}^{*})^{T}$

covariance matrix $C_{\Theta\Theta} = H^{-1}$

$$H_{ij} = -\frac{\partial^2 l(\boldsymbol{\theta} | \hat{\mathbf{x}})}{\partial \theta_i \partial \theta_j} \Big|_{\boldsymbol{\theta} = \boldsymbol{\theta}^*}$$

not just point estimates – full distribution information!

The Maximum Likelihood Method (MLM)

Let us consider the concrete compressive strength example

The log-likelihood function can be written as $l(\mathbf{\theta}|\hat{\mathbf{x}}) = n \cdot \ln\left(\frac{1}{\sqrt{2\pi}\theta_1}\right) - \frac{1}{2} \sum_{i=1}^n \frac{(\hat{x}_i - \theta_2)^2}{\theta_1^2}$

the minimum of which may be found by the solution of the following equations



The Maximum Likelihood Method (MLM)

Putting numbers into the solution we get:

$$\theta_1 = \sqrt{\frac{\sum_{i=1}^n (\hat{x}_i - \theta_2)^2}{n}} = \sqrt{\frac{367.19}{20}} = 4.05$$

Mean value of the standard deviation

$$\theta_2 = \frac{1}{n} \sum_{i=1}^n \hat{x}_i = \frac{653.3}{20} = 32.67$$

Mean value of the mean value



The Maximum Likelihood Method (MLM)

As mentioned we may also determine the covariance matrix:


Estimation of Distribution Parameters

The Maximum Likelihood Method (MLM)

We may also estimate the parameters completely numerically using Excel

Lets take a look !



Estimation of Distribution Parameters

Summary

Given that we have selected a model for the distribution i.e. a distribution family $f_x(x)$



we have to estimate the distribution parameters

- Method of Moments
- Maximum Likelihood Method

Estimation of Distribution Parameters

• Summary

Method of Moments provide point estimates of the parameters

- No information about the uncertainty with which the parameter estimates are associated.

Maximum Likelihood Method provide point estimates of the estimated parameters

- Full distribution information – normal distributed parameters, mean values and covariance matrix.

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Contents of Todays Lecture

- Short Summary of the Previous Lecture
- Overview of Estimation and Model Building
- Model Evaluation by Statistical Testing
 - The χ^2 goodness of fit test
 - The Kolmogorov-Smirnov goodness of fit test
 - Model comparison

Short Summary of the Previous Lecture

 We considered the problem of assessing the parameters of distributions based on observations/data

What did we learn?

We learned that parameters can be estimated using the

- Method of Moments
- Method of Maximum Likelihood

Short Summary of the Previous Lecture

• The Method of Moments (MoM) – point estimates

The principle behind the MoM is that we estimate the parameters such that the moments we can calculate based on the analytical expressions become equal to the sample moments.

$$m_{1} = \frac{1}{n} \sum_{i=1}^{n} \hat{x}_{i} \qquad \lambda_{1} = \int_{-\infty}^{\infty} x \cdot f_{X}(x \mid \mu, \sigma) dx$$

$$m_2 = \frac{1}{n} \sum_{i=1}^n \hat{x}_i^2 \qquad \lambda_2 = \int_{-\infty}^\infty x^2 \cdot f_X(x|\mu,\sigma) dx$$

This leads to *n* equations which have to be solved simultaneously where *n* is the number of parameters



Short Summary of the Previous Lecture

 The Method of Maximum Likelihood (MLM) – full distribution estimates

The principle behind the MLM is that we estimate the parameters such that the likelihood of the observations (data) is maximized)

$$L(\boldsymbol{\theta}|\hat{\mathbf{x}}) = \prod_{i=1}^{n} \frac{1}{\sigma\sqrt{2\pi}} \exp\left(-\frac{1}{2}\left(\frac{\hat{x}_{i}-\mu}{\sigma}\right)^{2}\right)$$

$$\min_{\boldsymbol{\theta}} \left(-L\left(\boldsymbol{\theta}|\hat{\mathbf{x}}\right)\right) \quad l(\boldsymbol{\theta}|\mathbf{x}) = \sum_{i=1}^{n} \log(f_{X}\left(\hat{x}_{i}|\boldsymbol{\theta}\right)$$

The MLM provides an extremely strong statistical tool!



Overview of Estimation and Model Building

Different types of information is used when developing engineering models

- subjective information
- frequentististic information



Let us assume that we have selected a distribution function as a model to describe an uncertain quantity



Now we want to validate our model selection – by means of statistical tests

Two different cases are considered – namely verification of

 Discrete distribution functions
CHI-Square (χ²) test

2: Continuous distribution functions

Kolmogorov Smirnov test







The CHI-square goodness of fit test

The idea behind the CHI-Square goodness of fit test is that the difference between predicted and observed/sample histograms should be small



The CHI-square goodness of fit test

We remember that a discrete cumulative distribution is given by:



The CHI-square goodness of fit test

Assuming that we sample a discrete random variable X*n* times the number of realizations of $X=x_i$ i.e. N_i is a binomial distributed random variable with expected value and variance given as:

 $E[N_i] = np(x_i) = N_{p,i}$ $Var[N_i] = np(x_i)(1 - p(x_i)) = N_{p,i}(1 - p(x_i))$

If the postulated model is correct and *n* large enough – Central Limit Theorem - the difference ε_i



Observed number of occurences at a given value

will be standard Normal distributed

The CHI-square goodness of fit test

By summing up the squared differences between the observed and the predicted histograms we get:



The CHI-square goodness of fit test

The idea is then to test – at a given significance level – α – if the sum of observed squared differences is plausible i.e.

Postulating the H_{θ} hypothesis that the assumed distribution function is not in gross contradiction with the observed data and formulating the operating rule such as the null hypothesis cannot be accepted if $\mathcal{E}_m^2 \ge \Delta$. The critical value Δ can be estimated such as:

$$P(\mathcal{E}_m^2 \ge \Delta) = \alpha$$

The alternate hypothesis H_1 is far less informative because it considers all other distribution functions than the assumed.

The CHI-square goodness of fit test

Consider as an example that we assume a Normal distribution with parameters not estimated from the available data

Mean:33 MpaStandard deviation:5 Mpa

The Normal distribution is a continuous probability density function but can easily be discretized



The CHI-square goodness of fit test

The postulated probability density function is discretized:



The CHI-square goodness of fit test

The observed and the predicted histograms may be compared



Due to a low number of samples in the lower interval the two lower intervals are "lumped"

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At the 5% significance level the CHI-Square distribution with 3-1=2 degree of freedom yields $\Delta = 5.99$ As 0.40987 is smaller than 5.99 the H_0 hypothesis cannot be rejected !

The CHI-square goodness of fit test

If one or more (m) of the parameters of the postulated distribution function had been assessed using the same data as used for the testing we must reduce the number of degrees of freedom accordingly i.e. n = k-1-m

Assuming that we had estimated the variance from the data but not the mean value we would have n=3-1-1=1



The CHI-square goodness of fit test

Assuming a postulated Normal distribution with

- μ = 33.00
- σ = 4.05

We get the following calculation sheet

Interval - x_j	Number of observed values $N_{o,j}$	Predicted probability $p(x_j)$	Predicted number of observations $N_{p,j} = 20p(x_j)$	Sample statistic Equation (5.26)	
0 -30	5	0.274253	5.485061	0.042896	
30-35	9	0.381169	7.623373	0.248591	
35-∞	6	0.344578	6.891566	0.115342	
			Sum	0.406829	

At the 5% significance level the CHI-Square distribution with 3-1-1 = 1 degrees of freedom yields $\Lambda = 3.84$ As 0.406829 is smaller than 3.84 the H_{θ} hypothesis cannot be rejected !

The Kolmogorov-Smirnov goodness of fit test

The idea behind the Kolmogorov-Smirnov test is that

If the postulated cumulative distribution function is in accordance with the observed data then the maximal difference between the observed and the predicted cumulative distribution functions should be small



The Kolmogorov-Smirnov goodness of fit test

The observed cumulative distribution function may be calculated from

$$F_o(x_i) = \frac{i}{n}$$

The following statistic has been proposed

$$\varepsilon_{\max} = \max_{i=1}^{n} \left[\left| F_o(x_i) - F_p(x_i) \right| \right] = \max_{i=1}^{n} \left[\left| \frac{i}{n} - F_p(x_i) \right| \right]$$

The Kolmogorov-Smirnov goodness of fit test

The Kolmogorov-Smirnov statistic may be assessed from



i	X _i	F _{xo} (x _i)	$F_{xp}(x_i)$	ε _i	
1	24.4	0.05	0.042716	0.007284	
2	27.6	0.1	0.140071	0.040071	
3	27.8	0.15	0.14917	0.00083	
4	27.9	0.2	0.153864	0.046136	
5	28.5	0.25	0.18406	0.06594	
6	30.1	0.3	0.280957	0.019043	
7	30.3	0.35	0.294598	0.055402	
8	31.7	0.4	0.397432	0.002568	
9	32.2	0.45	0.436441	0.013559	
10	32.8	0.5	0.484047	0.015953	
11	33.3	0.55	0.523922	0.026078	
12	33.5	0.6	0.539828	0.060172	
13	34.1	0.65	0.587064	0.062936	
14	34.6	0.7	0.625516	0.074484	
15	35.8	0.75	0.71226	0.03774	
16	35.9	0.8	0.719043	0.080957	
17	36.8	0.85	0.776373	0.073627	
18	37.1	0.9	0.793892	0.10610	
19	39.2	0.95	0.892512	0.057488	
20	39.7	1	0.909877	0.090123	

The Kolmogorov-Smirnov goodness of fit test

The Kolmogorov-Smirnov statistic is tabulated

	n											
α	1	5	10	15	20	25	30	40	50	60	70	80
0.01	0.9950	0.6686	0.4889	0.4042	0.3524	0.3166	0.2899	0.2521	0.2260	0.2067	0.1917	0.1795
0.05	0.9750	0.5633	0.4093	0.3376	0.2941	0.2640	0.2417	0.2101	0.1884	0.1723	0.1598	0.1496
0.1	0.9500	0.5095	0.3687	0.3040	0.2647	0.2377	0.2176	0.1891	0.1696	0.1551	0.1438	0.1347
0.2	0.9000	0.4470	0.3226	0.2659	0.2315	0.2079	0.1903	0.1654	0.1484	0.1357	0.1258	0.1179

For
$$n = 20$$
 and $a = 5\%$ we get 0.2941

compared to observed statisic 0.1061

The H_{θ} hypothesis cannot be rejected at the 5% significance level.

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Model comparison

Model verification by significance testing can be used to quantify the plausibility of a given model relative to given data (evidence)

Two cases have to be considered

- 1 it is shown that a model hypothesis cannot be rejected
- 2 it is shown that a model hypothesis can be rejected

What information is actually contained in these two cases ?

Model comparison

Given that the significance test shows that a model hypothesis cannot be rejected:

we must remember that other models could also be postulated – in fact it is often the case that several model hypothesis may pass testing !

Given that the significance test shows that a model hypothesis should be rejected:

it does not mean that the model necessary is bad – it may just say that the evidence is not strong enough to show it with significance – too little data !

Model comparison

If testing of two different model hypothesis both fall out positive i.e. both models are plausible we can compare the goodness of fit of the two models either by

- comparing the sample statistics directly could be misleading/inconclusive due to different number of degrees of freedom
- comparing the sample likelihoods

Model comparison

Consider the example with two different models

Model 1: N(33;5)

Parameters estimated not using data n=3-1=2

CHI-Square sample statistic = 0.40987

Sample likelihood \neq 0.8151

Model 2: N(33;4.05)Parameters estimated using data n=3-1-1=1

> CHI-Square sample statistic = 0.40683 Sample likelihood = 0.5236

Summary

The selection of appropriate probabilistic models may be supported by significance testing of the model hypothesis

The CHI-Square test is designed especially for discrete distribution functions

The Kolmogorov-Smirnov test is designed especially for continuous distribution functions

The goodness of fit of different model alternatives may be compared by comparing sample likelihood



Statistics and Probability Theory in

Civil, Surveying and Environmental Engineering

Prof. Dr. Michael Havbro Faber Swiss Federal Institute of Technology ETH Zurich, Switzerland



Contents of Todays Lecture

- Basics of Reliability Analysis
 - Short summary of previous lecture
 - The course at a glance
 - Failure events and basic random variables
 - Linear limit state functions and Normal distributed variables
 - Error propagation
 - Non-linear limit state functions
 - Monte-Carlo simulation

Summary of Previous Lecture

- Testing for goodness of fit
 - The $\chi 2$ goodness of fit test
 - The Kolmogorov-Smirnov goodness of fit test
- Model comparison



Summary of Previous Lecture

The CHI-square goodness of fit test

We test a ststistic constructed from the squared differences between the observed and the predicted histograms:



Summary of Previous Lecture

The Kolmogorov-Smirnov goodness of fit test

The observed cumulative distribution function may be calculated from:

$$F_o(x_i) = \frac{i}{n}$$



The following statistic is applied (tabularized):

$$\mathcal{E}_{\max} = \max_{i=1}^{n} \left[\left| F_o(x_i) - F_p(x_i) \right| \right] = \max_{i=1}^{n} \left[\left| \frac{i}{n} - F_p(x_i) \right| \right]$$


Summary of Previous Lecture

Model comparison

If testing of two different model hypothesis both fall out positive i.e. both models are plausible we can compare the goodness of fit of the two models either by

- comparing the sample statistics directly could be misleading/inconclusive due to different number of degrees of freedom
- comparing the sample likelihoods

The Course at a Glance



• Failure events and basic random variables

By a failure event we associate in principle an event of special interest e.g. :

- Loss of functionality
- Costs
- Loss of lives
- Damage to the environment



• Failure events and basic random variables

A failure event may conveniently be described in terms of a functional relationship

$$\mathbf{F} = \left\{ g(\mathbf{x}) \le 0 \right\}$$

Such a functional relationship is denoted a limit state function

g(x) ↑ Realizations of basic random variables

• The probability of an event

The probability of an event e.g. a failure event can be calculated by the following integral



• The probability of an event

The probability integral is in general non-trivial – can be multi-dimensional and can have a complicated integration domain

$$P_f = \int_{g(\mathbf{x}) \le 0} f_{\mathbf{X}}(\mathbf{x}) d\mathbf{x}$$

Classical nummerical integration techniques such as e.g. Simpson, Gauss or Schebyschev integration are not computationally efficient for dimensions larger than 5-6. Other apporaches are needed – which we will study further -

• Linear limit state functions and normal distributed basic variables

First we consider the case where the limit state function is linear in the random variables and the random variables are normally distributed

$$g(x) = a_0 + \sum_{i=1}^n a_i x_i$$

For the case where the random variables X are normal distributed the safety margin M is also normal distributed

$$M = a_0 + \sum_{i=1}^n a_i X_i$$

$$\mu_M = a_0 + \sum_{i=1}^n a_i \mu_{X_i}$$

$$\sigma_M^2 = \sum_{i=1}^n a_i^2 \sigma_{X_i}^2 + \sum_{i=1}^n \sum_{j=1, j \neq i}^n \rho_{ij} a_i a_j \sigma_{X_i} \sigma_{X_j}$$

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• Linear limit state functions and normal distributed basic variables

The probability of failure is then determined as

$$P_F = P(g(\mathbf{X}) \le 0) = P(M \le 0)$$

Which reduces to the determination of the standard normal probability distribution function

$$P_{F} = \Phi(\frac{0 - \mu_{M}}{\sigma_{M}}) = \Phi(-\beta) \quad \text{with} \quad \beta = \frac{\mu_{M}}{\sigma_{M}}$$

$$\int \sigma_{M}$$
Reliability or safety index

• Linear limit state functions and normal distributed basic variables



The safety margin

• Linear limit state functions and normal distributed basic variables

The reliability index β has a geometrical interpretation





Zero mean and unit variance

 Linear limit state functions and normal distributed basic variables

Example : Reliability of steel rod under tension loading

The resistance *R* and the max annual loading *S* are both assumed to be normal distributed

$$\mu_{R} = 350, \sigma_{R} = 35$$

$$\mu_s=200, \sigma_s=40$$

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r

S

 Linear limit state functions and normal distributed basic variables

Example : Reliability of steel rod under tension loading

The safety margin is thus normal distributed with parameters

$$\mu_M = 350 - 200 = 150$$

$$\sigma_{M} = \sqrt{35^2 + 40^2} = 53.15$$

The reliability index β becomes

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$$\beta = \frac{150}{53.15} = 2.84$$

 $P_F = \Phi(-2.84) = 2.4 \cdot 10^{-3}$

• The error accumulation law

In many engineering applications the accumulation of errors is a central question

Examples are :

- errors due to fabrication tolerances of building components
- errors in connection with surveying
- errors in connection with measurements performed in the laboratory

The error propagation law

Assume that the error ε can be written as a differentiable function of random variables i.e. :

$$\mathcal{E} = h(\mathbf{x})$$
 $\mathbf{x} = (x_1, x_2, ..., x_n)^T$ Vector of realization of basic random variables with parameters

$$\boldsymbol{\mu}_{\mathbf{X}} = (\mu_{X_1}, \mu_{X_2}, ..., \mu_{X_n})^T$$

$$Cov[X_i, X_j] = \rho_{ij}\sigma_{X_i}\sigma_{X_j}$$

Correlation coefficient

The idea is to linearize f(x)

$$\mathcal{E} \cong h(\mathbf{x}_0) + \sum_{i=1}^n (x_i - x_{i,0}) \frac{\partial f(\mathbf{x})}{\partial x_i} \bigg|_{\mathbf{x} = \mathbf{x}_0}$$

Standard deviation

First order partial derivative taken in $\mathbf{x} = \mathbf{x}_0$

The error propagation law

If we linearize the error function around the mean value of the random variables its expected value and variance becomes :

$$\varepsilon \simeq h(\mathbf{\mu}_{\mathbf{X}}) + \sum_{i=1}^{n} (x_{i} - \mu_{X_{i}}) \frac{\partial h(\mathbf{x})}{\partial x_{i}} \Big|_{\mathbf{x}=\mathbf{\mu}_{\mathbf{X}}}$$

$$E[\varepsilon] = h(\mathbf{\mu}_{\mathbf{X}})$$

$$Var[\varepsilon] = \sum_{i=1}^{n} \left(\frac{\partial h(\mathbf{x})}{\partial x_{i}} \Big|_{\mathbf{x}=\mathbf{\mu}_{\mathbf{X}}} \right)^{2} \sigma_{X_{i}}^{2} + \sum_{i=1}^{n} \sum_{j=1, j\neq i}^{n} \left(\frac{\partial h(\mathbf{x})}{\partial x_{i}} \Big|_{\mathbf{x}=\mathbf{\mu}_{\mathbf{X}}} \right) \left(\frac{\partial h(\mathbf{x})}{\partial x_{j}} \Big|_{\mathbf{x}=\mathbf{\mu}_{\mathbf{X}}} \right) \rho_{ij} \sigma_{X_{i}} \sigma_{X_{j}}$$

The mean value and the variance depends on the linearization point Swiss Federal Institute of Technology

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• Example : Error propagation in measurements

In order to estimate the length *c* i.e. the distance between the two points A and B the lengths *a* and *b* are measured **B**



due to measurement uncertainty in assessing *a* and *b* also the length of *c* will be associated with uncertainty and it is of interest to know the probability that the length of *c* will exceed 13.5

• Example : Error propagation in measurements

It is assumed that *a* and *b* can be modeled as normal distributed random variables with parameters



the statistical characteristics of *c* may be estimated through the error propagation law

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• Example : Error propagation in measurements

$$E[c] = \sqrt{\mu_a^2 + \mu_b^2}$$

$$Var[c] = \sum_{i=1}^n \left(\frac{\partial h(\mathbf{x})}{\partial x_i} \Big|_{\mathbf{x}=\mathbf{\mu}_{\mathbf{x}}} \right)^2 \sigma_{x_i}^2 = \frac{\mu_a}{\sqrt{\mu_a^2 + \mu_b^2}} \sigma_a^2 + \frac{\mu_b}{\sqrt{\mu_a^2 + \mu_b^2}} \sigma_b^2$$

$$E[c] = \sqrt{12.2^2 + 5.1^2} = 13.22$$

$$Var[c] = \frac{12.2^2}{12.2^2 + 5.1^2} 0.4^2 + \frac{5.1^2}{12.2^2 + 5.1^2} 0.3^2 = 0.15$$

$$P_f = P(13.5 - C \le 0) = \Phi(-\frac{13.5 - 13.22}{\sqrt{0.15}}) = 0.2349$$

• Non-linear limit state functions

Limit state functions are often non-linear

As seen from the error propagation law it is possible to linearize such limit state functions but the results will depend on the linearization point and on the formulation of the limit state function



- Non-linear limit state functions
 - Limit state functions are often non-linear



Hasofer and Lind suggested to linearize in the point where the limit state function is zero and closest to the origin in normal distributed space

Non-linear limit state functions

The identification of the reliability index may be performed by solving an optimization problem



$$\beta = \min_{\mathbf{u} \in \{g(\mathbf{u})=0\}} \sqrt{\sum_{i=1}^{n} u_i^2}$$



Non-linear limit state functions

The optimization problem may be solved using the following iteration scheme

$$\alpha_{i} = \frac{-\frac{\partial g}{\partial u_{i}}(\beta \alpha)}{\left[\sum_{j=1}^{n} \frac{\partial g}{\partial u_{i}}(\beta \alpha)^{2}\right]^{1/2}}, \quad i = 1, 2, ..n$$

$$g(\beta \alpha_1, \beta \alpha_2, \dots \beta \alpha_n) = 0$$



Provided that the limit state function is differentiable !

In summary the iteration follows the following steps

- 1) the linearization point is chosen as $u^* = \beta \alpha$
- 2) the Normal vector to the limit state function is determined in the linearization point
- 3) the reliability index β is calculated from
- 4) the new linearization point is
- 5) continue with step 2) until convergence in β





$$\alpha_{i} = \frac{-\frac{\partial g}{\partial u_{i}}(\beta \alpha)}{\left[\sum_{j=1}^{n} \frac{\partial g}{\partial u_{i}}(\beta \alpha)^{2}\right]^{1/2}}, \quad i = 1, 2, ..n$$
$$g(\beta \alpha_{1}, \beta \alpha_{2}, ..., \beta \alpha_{n}) = 0$$

$$u^* = (\beta \alpha_1, \beta \alpha_2, \dots \beta \alpha_n)^T$$

Non-linear safety margins



• Non-linear safety margins







Linearization of limit state function in X¹























Example : Reliability of steel rod

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Limit state function

Yield stress

$$g(\mathbf{x}) = r \cdot a - s \leftarrow \text{Load}$$
Cross sectional area

a S

 $\mathcal{G}, \mathcal{G}_A$

it is assumed that R, S and A are normal distributed random variables

$$U_{R} = \frac{R - \mu_{R}}{\sigma_{R}} \qquad U_{S} = \frac{S - \mu_{S}}{\sigma_{S}} \qquad U_{A} = \frac{A - \mu_{A}}{\sigma_{A}} \qquad \mu_{R} = 350, \sigma_{R} = 35$$
$$\mu_{S} = 1500, \sigma_{S} = 300$$
$$\mu_{A} = 10, \sigma_{A} = 2$$

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Example : Reliability of steel rod ٠

We can now write the limit state function in terms of *u*-variables



$$r \qquad a \qquad s$$
$$g(u) = (u_R \sigma_R + \mu_R)(u_A \sigma_A + \mu_A) - (u_S \sigma_S + \mu_S)$$

$$= (35u_R + 350)(u_A + 10) - (300u_S + 1500)$$

= 350u_R + 350u_A - 300u_S + 35u_Ru_A + 2000

• Example : Reliability of steel rod

1

The reliability index β may be found by iteration

$$\alpha_{R} = -\frac{1}{k}(350+35\beta\alpha_{A})$$

$$\alpha_{A} = -\frac{1}{k}(350+35\beta\alpha_{R}) \qquad k = \sqrt{\alpha_{R}^{2} + \alpha_{A}^{2} + \alpha_{S}^{2}}$$

$$\alpha_{S} = \frac{300}{k}$$

$$\beta = \frac{-2000}{350\alpha_{R} + 350\alpha_{A} - 300\alpha_{S} + 35\beta\alpha_{R}\alpha_{A}}$$

$$\frac{|\text{teration} \quad \text{Start} \quad 1 \quad 2 \quad 3 \quad 4 \quad 5}{\frac{\beta}{2} \quad 3.000 \quad 3.6719 \quad 3.7399 \quad 3.7444 \quad 3.7448 \quad 3.7448}{\alpha_{R}} \quad 0.5800 \quad 0.5701 \quad 0.5612 \quad 0.5611 \quad 0.5610 \quad 0.5610} \\ \alpha_{S} = 0.5800 \quad 0.5916 \quad 0.6084 \quad 0.6086 \quad 0.6087 \quad 0.6087}$$

$$= 350u_{R} + 350u_{A} - 300u_{S} + 35u_{R}u_{A} + 2000 \quad 0.5701 \quad 0.5612 \quad 0.5611 \quad 0.5610 \quad 0.5610}{\alpha_{S} \quad 0.5800 \quad 0.5916 \quad 0.6084 \quad 0.6086 \quad 0.6087 \quad 0.6087}$$

Monte Carlo Simulation
 The probability integration
 problem may be solved by
 Monte Carlo simulation

m realizations of the vector X are produced
 for every realization the limit state function is calculated
 the realizations for which the limit state function is equal to or less than zero are counted
 The probability of failure is estimated as

$$P_f = \int_{\Omega_f = \{g(\mathbf{x}) \le 0\}} f_{\mathbf{X}}(\mathbf{x}) d\mathbf{x}$$

 n_{f}



Z is a random number uniformly distributed between 0 and 1
Basics of Reliability Analysis

Monte Carlo Simulation

m random realizations of *R* and *S* are generated and the number of realizations n_f occuring in the failure space are counted n_f

The probability of failure p_f is then

$$p_f = \frac{n_f}{m}$$



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A Summary of the Lecture

Graphical/numerical interpretation of data



Verification and testing of models



Basic probability theory



Bayesian modeling



Distribution functions moments and extremes



Basic reliability analysis



Modeling and Description of data



Basic decision analysis



- Introduction to Decision Theory
 - The problem
 - The decision tree
 - Prior decision analysis
 - Posterior decision analysis
 - Pre-posterior decision analysis

• The basic engineering problem



Approach

- Formulation of the decision problem
 - The decision maker and the preferences of the decision maker must be identified
 - Mapping of the decision process
 - All the possible decision alternatives must be identified
 - Identification of the contributing uncertainties
- Identification of potential consequences and their utility (cost/benefit)
- Assessment of the probabilities of the consequences
- Comparison of the different decision alternatives based on their expected utilities
- Final decision making and reporting of the assumptions underlying the selected alternative



Assignment of utility

- The assignment of utility must reflect the preferences of the decision maker
- Utility functions may be defined as linear functions in monetary unity
- It is important to include all monetary consequences in the utility function

$$u(a_i) = \sum_{j=1}^n p_j \cdot u(K_j)$$

- $u(a_i)...$ U tility (cost/bene fit) associated with action a_i
- $p_j \cdot u(K_j)$... Expected utility associated with consequence K_j
- p_i ... Probabilit y of the occurrence of the consequence K_i
- $u(K_j)$... Ut ility associated with the consequence K_j

 K_j ... A potential consequence associated with the action a_i Swiss Federal Institute of Technology

The different types of decision analysis

- Prior
- Posterior
- Pre-posterior

Illustrated on an example :

Question : What pile length should be applied ?

Alternatives : a_0 : Choose a 40 ft pile a_1 : Choose a 50 ft pile States of nature (depth to rock bed) 0 : Rock bed in 40 ft 1 : Rock bed at 50 ft



p=0.70 u = 0120 **Prior Analysis** _p=0.30 a_0 u = 400 (Pile is spliced) $P'[q_0] = 0.70$ $P'[q_1] = 0.30$ p=0.70 u = 100 (Pile is cut) a_1 p=0.3070 11 = 0The expected utility is calculated to be equal to $E'[u] = \min\{u[a_0], u[a_1]\}$ $= \min\{P'[\theta_0] \times u[\theta_0|a_0] + P'[\theta_1] \times u[\theta_1|a_0],$ $P'[\theta_0] \times u[\theta_0|a_1] + P'[\theta_1] \times u[\theta_1|a_1]$ $= \min\{0.7 \times 0 + 0.3 \times 400, 0.7 \times 100 + 0.3 \times 0\}$ $= \min\{120,70\} = 70 \implies \text{Decision for } a_1 \text{ (50ft Pile)}$



 \Rightarrow Choice of pile a_1 (50ft Pfahl)

Posterior Analysis

$$P''(\theta_i) = \frac{P[z_k | \theta_i] P'[\theta_i]}{\sum_j P[z_k | \theta_j] P'[\theta_j]}$$
probability of θ_i

$$= \begin{pmatrix} \text{Normalizing} \\ \mathbf{x} \end{pmatrix} \begin{pmatrix} \text{Samplelikelihood} \\ \mathbf{x} \end{pmatrix} \begin{pmatrix} \text{prior probability} \end{pmatrix}$$

Posterior

with given sample outcome $\int constant \int d given \theta \int d of \theta$

Posterior Analysis Prior Likelihood Posterior $\overline{\mathcal{A}}$ Posterior Likelihood Prior Likelihood Prior ∕∖ Posterior

$P''(\theta_i) = \frac{P[z_k | \theta_i] P'[\theta_i]}{\sum_{i} P[z_k | \theta_j] P'[\theta_j]}$

Posterior Analysis

Ultrasonic tests to determine the depth to bed rock

True state	θ	θι
Test result	40 ft – depth	50 ft – depth
z_0 - 40 ft indicated	0.6	0.1
z_1 - 50 ft indicated	0.1	0.7
z_2 - 45 ft indicated	0.3	0.2

Likelihoods of the different indications/test results given the various possible states of nature – ultrasonic test methods



Decision Analysis in Engineering $P''(\theta_i) = \frac{P[z_k | \theta_i] P'[\theta_i]}{\sum_j P[z_k | \theta_j] P'[\theta_j]}$ Posterior Analysis It is assumed that a test gives a 45 ft indication

$$P''[\theta_0] = P[\theta_0|z_2] \propto P[z_2|\theta_0]P[\theta_0] = 0.3 \ x \ 0.7 = 0.21$$
$$P''[\theta_1] = P[\theta_1|z_2] \propto P[z_2|\theta_1]P[\theta_1] = 0.2 \ x \ 0.3 = 0.06$$

$$P''\left[\theta_{0}|z_{2}\right] = \frac{0.21}{0.21 + 0.06} = 0.78$$
$$P''\left[\theta_{1}|z_{2}\right] = \frac{0.06}{0.21 + 0.06} = 0.22$$

Posterior Analysis

Test result indicates 45ft to rock bed





 $= \min\{P''[\theta_0] \times 0 + P''[\theta_1] \times 400, P''[\theta_0] \times 100 + P''[\theta_1] \times 0\}$ $= \min\{0.78 \times 0 + 0.22 \times 400, 0.78 \times 100 + 0.22 \times 0\}$

 $= \min\{88, 78\} = 78$

 $\implies \text{Choice of alternative } a_1 \\ \text{(50ft Pile)}$

Pre-posterior Analysis

$$E[u] = \sum_{i=1}^{n} P'[z_i] \times E''[u|z_i] = \sum_{i=1}^{n} P'[z_i] \times \min_{j=1,m} \{E''[u(a_j)|z_i]\}$$
$$P'[z_i] = P[z_i|\theta_0] \times P'[\theta_0] + P[z_i|\theta_1] \times P'[\theta_1]$$

$$P'[z_0] = P[z_0|\theta_0] \times P'[\theta_0] + P[z_0|\theta_1] \times P'[\theta_1] = 0.6 \times 0.7 + 0.1 \times 0.3 = 0.45$$

$$P'[z_1] = P[z_1|\theta_0] \times P'[\theta_0] + P[z_1|\theta_1] \times P'[\theta_1] = 0.1 \times 0.7 + 0.7 \times 0.3 = 0.28$$

$$P'[z_2] = P[z_2|\theta_0] \times P'[\theta_0] + P[z_2|\theta_1] \times P'[\theta_1] = 0.3 \times 0.7 + 0.2 \times 0.3 = 0.27$$

Pre-posterior Analysis

$$E''[u|z_0] = \min_{j} \{E''[u(a_j)|z_0]\} =$$



Pre-posterior Analysis

$$E''[u|z_{1}] = \min_{j} \{E''[u(a_{j})|z_{1}]\} = \frac{a_{0}}{do \text{ nothing splicing cutting do nothing}}$$

$$\min_{i} \{P''[\theta_{0}|z_{1}] \times 0 + P''[\theta_{1}|z_{1}] \times 400, P''[\theta_{0}|z_{1}] \times 100 + P''[\theta_{1}|z_{1}] \times 0\}$$

$$\min_{i} \{0.25 \times 0 + 0.75 \times 400, 0.25 \times 100 + 0.75 \times 0\} =$$

 $0.25 \times 100 + 0.75 \times 0 = 25$

Pre-posterior Analysis

The minimum expected costs based on pre-posterior decision analysis – not including costs of experiments

$$E[u] = \sum_{i=1}^{n} P'[z_i] \times E''[u|z_i] = 28 \times 0.45 + 25 \times 0.28 + 78 \times 0.27 = 40.66$$

Allowable costs for the experiment

$$E'[u] - E[u] = 70.00 - 40.66 = 29.34$$

