# The Finite Element Method for the Analysis of Linear Systems



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## **Contents of Today's Lecture**

- Motivation, overview and organization of the course
- Introduction to the use of finite element
  - Physical problem, mathematical modeling and finite element solutions
  - Finite elements as a tool for computer supported design and assessment
- Basic mathematical tools



Motivation

#### In this course we are focusing on the assessment of the response of engineering structures





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Motivation

What we would like to establish is the response of a structure subject to "loading".

The Method of Finite Elements provides a framework for the analysis of such responses – however for very general problems.

The Method of Finite Elements provides a very general approach to the approximate solutions of differential equations.

In the present course we consider a special class of problems, namely:

Linear quasi-static systems, no material or geometrical or boundary condition non-linearities and also no inertia effect!



Organisation

The lectures will be given by:

M. H. Faber

**Exercises will be organized/attended by:** 

J. Qin

By appointment, HIL E13.1



#### Organisation

PowerPoint files with the presentations will be uploaded on our homepage one day in advance of the lectures

http://www.ibk.ethz.ch/fa/education/ss\_FE

The lecture as such will follow the book:

"Finite Element Procedures" by K.J. Bathe, Prentice Hall, 1996

#### • Overview

Date 20.02.2009	Pages 1-51	Subject Introduction to the use of finite elements, basic mathematical tools
27.02.2009	105-147	Basic concepts of engineering analysis
06.03.2009	149-200	Displacement based method of finite elements
13.03.2009		Formulation of finite elements
20.03.2009	455-484	Implementation
27.03.2009	338-340, 363-376	Isoparametric finite element matrices: truss element, triangular element

#### • Overview

03.04.2009	342-363, 386-388	Quadrilateral elements; Element matrices in global coordinates
24.04.2009	397-420	Beam elements and axisymmetric shell elements
08.05.2009	420-436	Plate elements
15.05.2009	437-450	Shell elements
22.05.2009	695-741	Solution of equilibrium equations in static analysis
29.05.2009	225-259	Convergence, compatibility, completeness, accuracy of the method of finite elements; Outlook







• *Reliability* of a mathematical model

The chosen mathematical model is reliable if the required response is known to be predicted within a selected level of accuracy measured on the response of a very comprehensive mathematical model

• *Effectiveness* of a mathematical model

The most effective mathematical model for the analysis is surely that one which yields the required response to a sufficient accuracy and at least costs

• Example

Complex physical problem modeled by a simple mathematical model  $\rightarrow$ 



• Example

Detailed reference model – 2D plane stress model – for FEM analysis  $\frac{\partial \tau_{xx}}{\partial r} + \frac{\partial \tau_{xy}}{\partial v} = 0$ 



$$\begin{cases} \frac{\partial \tau_{xx}}{\partial x} + \frac{\partial \tau_{xy}}{\partial y} = 0\\ \frac{\partial \tau_{yx}}{\partial x} + \frac{\partial \tau_{yy}}{\partial y} = 0 \end{cases}$$
 in domain of bracket

 $\tau_{nn} = 0$ ,  $\tau_{nt} = 0$  on surfaces except at point B and at imposed zero displacements Stress-strain relation:

 $\begin{bmatrix} \tau_{xx} \\ \tau_{yy} \\ \tau_{xy} \end{bmatrix} = \frac{E}{1 - v^2} \begin{bmatrix} 1 & v & 0 \\ v & 1 & 0 \\ 0 & 0 & \frac{1 - v}{2} \end{bmatrix} \begin{bmatrix} \varepsilon_{xx} \\ \varepsilon_{yy} \\ \gamma_{xy} \end{bmatrix}$ Strain-displacement relation:  $\varepsilon_{xx} = \frac{\partial u}{\partial x}; \varepsilon_{yy} = \frac{\partial v}{\partial y}; \gamma_{xy} = \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x}$ 

• Example

**Comparison between simple and more refined model results** 

M = WL= 27,500 Ncm $\delta\Big|_{at \ load \ W} = \frac{1}{3} \frac{W(L+r_N)^3}{EI} + \frac{W(L+r_N)}{\frac{5}{6}AG}$ = 0.053 cm**Reliability and efficiency**  $\delta|_{at \ load \ W} = 0.064 cm$ may be quantified!  $M|_{x=0} = 27,500 Ncm$ 



#### Observations

Choice of mathematical model must correspond to desired response measures

The most effective mathematical model delivers reliable answers with the least amount of efforts

Any solution (also FEM) of a mathematical model is limited to information contained in the model – bad input – bad output

Assessment of accuracy is based on comparisons with results from very comprehensive models – however, in practice often based on experience



#### Observations

Sometimes the chosen mathematical model results in problems such as singularities in stress distributions

The reason for this is that simplifications have been made in the mathematical modeling of the physical problem

Depending on the response which is really desired from the analysis this may be fine – however, typically refinements of the mathematical model will solve the problem



Finite elements as a tool for computer supported design and assessment

FEM forms a basic tool framework in research and applications covering many different areas

- Fluid dynamics
- Structural engineering
- Aeronautics
- Electrical engineering
- etc.





- Finite elements as a tool for computer supported design and assessment
  - The practical application necessitates that solutions obtained by FEM are reliable and efficient
  - however

also it is necessary that the use of FEM is *robust* – this implies that minor changes in any input to a FEM analysis should not change the response quantity significantly

Robustness has to be understood as directly related to the desired type of result – response

• Vectors and matrices

 $\mathbf{A}\mathbf{x} = \mathbf{b}$ 



$$\mathbf{x} = \begin{bmatrix} x_1 \\ x_2 \\ \vdots \\ x_n \end{bmatrix}, \ \mathbf{b} = \begin{bmatrix} b_1 \\ b_2 \\ \vdots \\ b_m \end{bmatrix}$$

 $\mathbf{A}^{T}$  is the transpose of  $\mathbf{A}$ if  $\mathbf{A} = \mathbf{A}^{T}$  there is m = n (square matrix) and  $a_{ij} = a_{ji}$  (symmetrical matrix)

$$\mathbf{I} = \begin{bmatrix} 1 & 0 & 0 & \cdots & 0 \\ 0 & 1 & 0 & \cdots & 0 \\ 0 & 0 & 1 & \cdots & 0 \\ \vdots & \vdots & \vdots & \ddots & \vdots \\ 0 & 0 & 0 & \cdots & 1 \end{bmatrix}$$
 is a unit matrix

Banded matrices

#### symmetric banded matrices

 $a_{ij} = 0$  for  $j > i + m_A$ ,  $2m_A + 1$  is the bandwidth

$$\mathbf{A} = \begin{bmatrix} 3 & 2 & 1 & 0 & 0 \\ 2 & 3 & 4 & 1 & 0 \\ 1 & 4 & 5 & 6 & 1 \\ 0 & 1 & 6 & 7 & 4 \\ 0 & 0 & 1 & 4 & 3 \end{bmatrix} \qquad m_A = 2$$

Banded matrices and skylines



• Matrix equality

 $\mathbf{A}(m \times p) = \mathbf{B}(n \times q) \text{ if and only if}$  m = n, p = q,and  $a_{ij} = b_{ij}$ 

• Matrix addition

 $\mathbf{A}(m \times p), \mathbf{B}(n \times q)$  can be added if and only if m = n, p = q, and if  $\mathbf{C} = \mathbf{A} + \mathbf{B}$ , then  $c_{ij} = a_{ij} + b_{ij}$  Page 23



• Matrix multiplication with a scalar

A matrix **A** multiplied by a scalar c by multiplying all elements of **A** with c

 $\mathbf{B} = c\mathbf{A}$  $b_{ij} = ca_{ij}$ 



• Multiplication of matrices

Two matrices  $\mathbf{A}(p \times m)$  and  $\mathbf{B}(n \times q)$  can be multiplied only if m = n

$$\mathbf{C} = \mathbf{B}\mathbf{A}$$
$$c_{ij} = \sum_{r=1}^{m} a_{ir} b_{rj}, \ \mathbf{C}(p \times q)$$



#### **Multiplication of matrices**

The commutative law does not hold, i.e. AB = CB does not imply that A = C

 $AB \neq BA$ , unless A and B commute

The distributive law hold, i.e.

 $\mathbf{E} = (\mathbf{A} + \mathbf{B})\mathbf{C} = \mathbf{A}\mathbf{C} + \mathbf{B}\mathbf{C}$ 

The associative law hold, i.e.

G = (AB)C = A(BC) = ABC

however does hold for special cases (e.g. for  $\mathbf{B} = \mathbf{I}$ )

Special rule for the transpose of matrix products

$$(\mathbf{A}\mathbf{B})^T = \mathbf{B}^T \mathbf{A}^T$$



• The inverse of a matrix

The inverse of a matrix A is denoted  $A^{-1}$ 

if the inverse matrix exist then there is:

 $\mathbf{A}\mathbf{A}^{-1} = \mathbf{A}^{-1}\mathbf{A} = \mathbf{I}$ 

The matrix A is said to be non-singular

The inverse of a matrix product:

$$\left(\mathbf{AB}\right)^{-1} = \mathbf{B}^{-1}\mathbf{A}^{-1}$$

#### • Sub matrices

A matrix **A** may be sub divided as:

$$\mathbf{A} = \begin{bmatrix} a_{11} & a_{12} & a_{13} \\ a_{21} & a_{22} & a_{23} \\ \hline a_{31} & a_{32} & a_{33} \end{bmatrix}$$

$$\mathbf{A} = \begin{bmatrix} \overline{a_{11}} & \overline{a_{12}} \\ \overline{a_{21}} & \overline{a_{22}} \end{bmatrix}$$

• Trace of a matrix

The trace of a matrix  $A(n \times n)$  is defined through:

$$tr(\mathbf{A}) = \sum_{i=1}^{n} a_{ii}$$

• The determinant of a matrix

The determinant of a matrix is defined through the recurrence formula

$$\det(\mathbf{A}) = \sum_{j=1}^{n} (-1)^{1+j} a_{1j} \det(\mathbf{A}_{1j})$$

where  $\mathbf{A}_{1j}$  is the  $(n-1) \times (n-1)$  matrix obtained by eliminating the 1<sup>st</sup> row and the *j*<sup>th</sup> column from the matrix **A** and where there is

if 
$$\mathbf{A} = [a_{11}]$$
, det  $\mathbf{A} = a_{11}$ 

#### • The determinant of a matrix

It is convenient to decompose a matrix **A** by the so-called Cholesky decomposition

$$\mathbf{A} = \mathbf{L}\mathbf{D}\mathbf{L}^{T} \qquad \qquad \mathbf{L} = \begin{bmatrix} 1 & 0 & 0 \\ l_{21} & 1 & 0 \\ l_{31} & l_{32} & 1 \end{bmatrix}$$

where **L** is a lower triangular matrix with all diagonal elements equal to 1 and **D** is a diagonal matrix with components  $d_{ii}$  then the determinant of the matrix **A** can be written as

$$\det \mathbf{A} = \prod_{i=1}^{n} d_{ii}$$

Tensors



Let the Cartesian coordinate frame be defined by the unit base vectors  $\mathbf{e}_i$ 

A vector **u** in this frame is given by

$$\mathbf{u} = \sum_{i=1}^{3} u_i \mathbf{e}_i$$

simply we write

 $\mathbf{u} = u_i \mathbf{e}_i$ 

*i* is called a dummy index or a free index

#### • Tensors

An entity is called a tensor of first order if it has 3 components  $\xi_i$  in the unprimed frame and 3 components  $\xi'_i$  in the primed frame, and if these components are related by the characteristic law

$$\xi_i' = p_{ik}\xi_i$$

where 
$$p_{ik} = \cos(\mathbf{e}_i, \mathbf{e}_k)$$

In the matrix form, it can be written as

$$\xi' = P\xi$$

#### Tensors

An entity is called a second-order tensor if it has 9 components  $t_{ij}$  in the unprimed frame and 9 components  $t'_{ij}$  in the primed frame, and if these components are related by the characteristic law

$$t_{ij} = p_{ik} p_{jl} t_{kl}$$