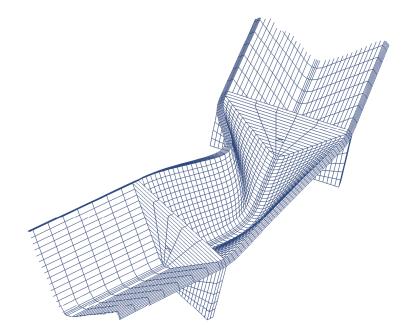


# The Finite Element Method for the Analysis of Non-Linear and Dynamic Systems



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Method of Finite Elements II

# **Contents of Today's Lecture**

• Solution of Equilibrium Equations in Dynamic Analysis

**Transformation Methods** 

- The Jacobi Method
- The Generalized Jacobi Method
- The Householders QR Inverse Iteration Solution (HQRI)
  - The QR iteration
  - Calculation of eigenvectors

#### **The Objective - Mode Superposition**

Modal Generalized Displacements

The direct integration methods necessitate that the finite element equations are evaluated for each time step

The bandwidth of the matrixes M, C and K depend on the numbering of the finite element nodal points

In principle we could try to rearrange the nodal point numbering but this approach is cumbersome and has limitations

Instead we transform the equations into a form which in terms of numerical effort is less expensive - by a change of basis



• Change of Basis to Modal Coordinates

#### The following transformation is introduced:

 $\mathbf{U}(t) = \mathbf{P}\mathbf{X}(t)$ 

P: *n* x *n* square matrixX(*t*): time dependent vector of order n

 $\tilde{\mathbf{M}}\ddot{\mathbf{X}}(t) + \tilde{\mathbf{C}}\dot{\mathbf{X}}(t) + \tilde{\mathbf{K}}\mathbf{X}(t) = \tilde{\mathbf{R}}(t)$  $\tilde{\mathbf{M}}\ddot{\mathbf{X}}(t) = \mathbf{P}^T\mathbf{M}\mathbf{P}, \quad \tilde{\mathbf{C}}\dot{\mathbf{X}}(t) = \mathbf{P}^T\mathbf{C}\mathbf{P}, \quad \tilde{\mathbf{K}}\mathbf{X}(t) = \mathbf{P}^T\mathbf{K}\mathbf{P}, \quad \tilde{\mathbf{R}}(t) = \mathbf{P}^T\mathbf{R}$ 



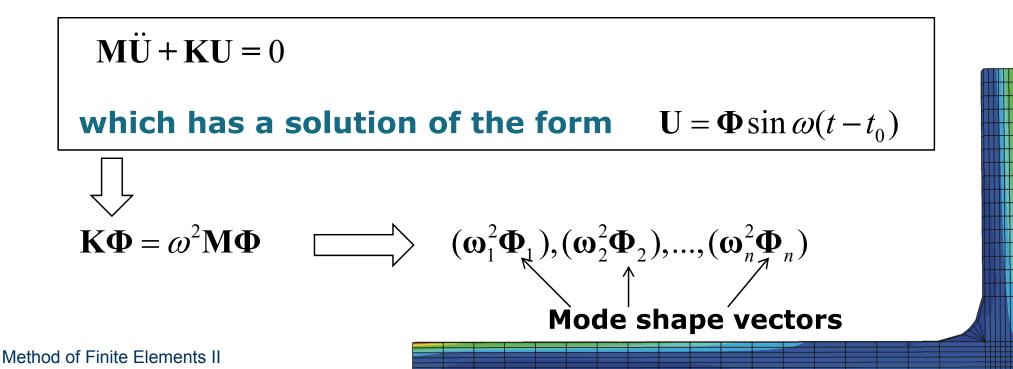
#### The Objective - Mode Superposition $\tilde{M}\ddot{X}(t) + \tilde{C}\dot{X}(t) + \tilde{K}X(t) = \tilde{R}(t)$

 $\tilde{\mathbf{M}}\ddot{\mathbf{X}}(t) = \mathbf{P}^T\mathbf{M}\mathbf{P}, \quad \tilde{\mathbf{C}}\dot{\mathbf{X}}(t) = \mathbf{P}^T\mathbf{C}\mathbf{P}, \quad \tilde{\mathbf{K}}\mathbf{X}(t) = \mathbf{P}^T\mathbf{K}\mathbf{P}, \quad \tilde{\mathbf{R}}(t) = \mathbf{P}^T\mathbf{R}$ 

• Change of Basis to Modal Coordinates

**The question is – how to choose P?** 

A good choice is to take basis in the free vibration solution – neglecting damping, i.e.:





#### **The Objective - Mode Superposition**

• Change of Basis to Modal Coordinates

Any of the solutions  $(\boldsymbol{\omega}_1^2 \boldsymbol{\Phi}_1), (\boldsymbol{\omega}_2^2 \boldsymbol{\Phi}_2), ..., (\boldsymbol{\omega}_n^2 \boldsymbol{\Phi}_n)$ satisfy  $M\ddot{U} + KU = 0$ 

The *n* solutions may be written as:  $\mathbf{K}\boldsymbol{\Phi} = \mathbf{M}\boldsymbol{\Phi}\boldsymbol{\Omega}^2, \quad \boldsymbol{\Phi}^T\mathbf{K}\boldsymbol{\Phi} = \boldsymbol{\Omega}^2; \quad \boldsymbol{\Phi}^T\mathbf{M}\boldsymbol{\Phi} = \mathbf{I}$ 

with: 
$$\boldsymbol{\Phi} = [\boldsymbol{\Phi}_1, \boldsymbol{\Phi}_2, ..., \boldsymbol{\Phi}_n]; \quad \boldsymbol{\Omega}^2 = \begin{bmatrix} \omega_1^2 & & \\ & \omega_n^2 & \\ & & \ddots \end{bmatrix}$$

 $\omega_n^2$ 



#### **The Objective - Mode Superposition**

#### • Change of Basis to Modal Coordinates

**Now using**  $U(t) = \Phi X(t)$ 

in

$$\tilde{\mathbf{M}}\ddot{\mathbf{X}}(t) + \tilde{\mathbf{C}}\dot{\mathbf{X}}(t) + \tilde{\mathbf{K}}\mathbf{X}(t) = \tilde{\mathbf{R}}(t)$$
  
$$\tilde{\mathbf{M}}\ddot{\mathbf{X}}(t) = \mathbf{P}^T\mathbf{M}\mathbf{P}, \quad \tilde{\mathbf{C}}\dot{\mathbf{X}}(t) = \mathbf{P}^T\mathbf{C}\mathbf{P}, \quad \tilde{\mathbf{K}}\mathbf{X}(t) = \mathbf{P}^T\mathbf{K}\mathbf{P}, \quad \tilde{\mathbf{R}}(t) = \mathbf{P}^T\mathbf{R}$$

#### we get

$$\ddot{\mathbf{X}}(t) + \mathbf{\Phi}^T \mathbf{C} \mathbf{\Phi} \dot{\mathbf{X}}(t) + \mathbf{\Omega}^2 \mathbf{X}(t) = \mathbf{\Phi}^T \mathbf{R}(t)$$

with  ${}^{0}\mathbf{X} = \mathbf{\Phi}^{T}\mathbf{M}^{0}\mathbf{U}; \qquad {}^{0}\dot{\mathbf{X}} = \mathbf{\Phi}^{T}\mathbf{M}^{0}\dot{\mathbf{U}}$ 

Introduction

We need to perform the transformations  $\Phi^T \mathbf{K} \Phi = \Lambda$  $\Phi^T \mathbf{M} \Phi = \mathbf{I}$ 

The transformation may be pursued by iteration

#### Introduction

The aim being to select  $\mathbf{P}_k$  such as to bring  $\mathbf{K}_k, \mathbf{M}_k$  closer to diagonal form i.e. :

$$\mathbf{K}_{k+1} \to \Lambda, \quad \mathbf{M}_{k+1} \to \mathbf{I}, \qquad k \to \infty$$

whereby there is:

 $\mathbf{\Phi} = \mathbf{P}_1 \cdot \mathbf{P}_2 \dots \mathbf{P}_l$ 

in practice we don't need convergence to

$$\mathbf{K}_{k+1} \to \Lambda, \quad \mathbf{M}_{k+1} \to \mathbf{I}, \qquad k \to \infty$$

Method of Finite Elements II



Introduction

ETH

In practice we don't need convergence to  $\Lambda$ , I only to diagonal form;

$$\mathbf{K}_{k+1} \to diag(K_r), \quad \mathbf{M}_{k+1} \to diag(M_r), \quad k \to \infty$$

If *I* is the last iteration there is:

$$\boldsymbol{\Lambda} = diag(\frac{K_r^{(l+1)}}{M_r^{(l+1)}})$$
$$\boldsymbol{\Phi} = \mathbf{P}_1 \cdot \mathbf{P}_2 \dots \mathbf{P}_l \ diag(\frac{1}{\sqrt{M_r^{(l+1)}}})$$

Introduction

Based on these consideration a number of iteration schemes have been proposed here we will consider the

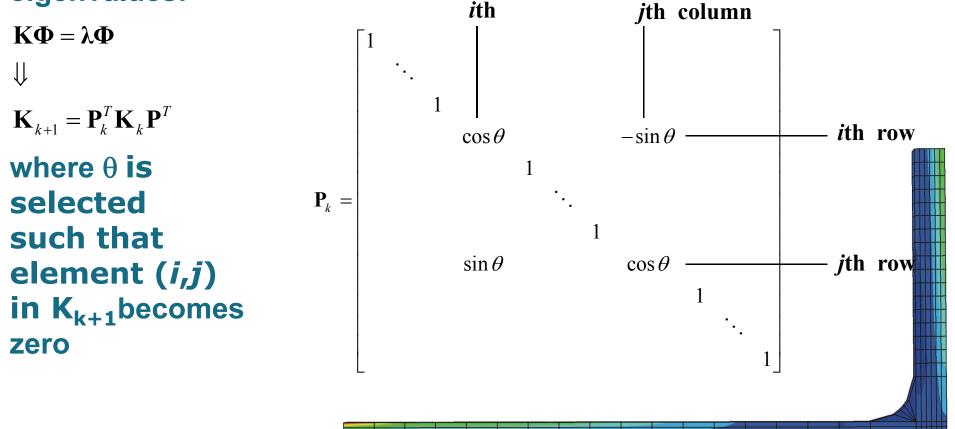
Jacobi

and the

Householder-QR-method

• The Jacobi Method (M = I)

This method can be applied to calculate negative, zero and positive eigenvalues!





$$\mathbf{K}_{k+1} = \mathbf{P}_k^T \mathbf{K}_k \mathbf{P}^T$$

#### • The Jacobi Method

# where $\theta$ is selected such that element (i,j) in $K_{k+1}$ becomes zero, i.e.:

$$\tan 2\theta = \frac{2k_{ij}^{(k)}}{k_{ii}^{(k)} - k_{jj}^{(k)}} \quad \text{for} \quad k_{ii}^{(k)} \neq k_{jj}^{(k)}$$
$$\theta = \frac{\theta}{4} \quad \text{for} \quad k_{ii}^{(k)} = k_{jj}^{(k)}$$

this zeroing can be performed for any (i,j) but after a new element is zeroed the other become non-zero again

$$\mathbf{K}_{k+1} = \mathbf{P}_k^T \mathbf{K}_k \mathbf{P}^T$$

• The Jacobi Method

A strategy is therefore to always zero the off diagonal elements furthest away from the diagonal – but this is time consuming (the searching process)

Another approach is simply to systematically go through all elements - sweep after sweep

 but this will repeatedly lead to zeroing of elements which are already almost equal to zero

$$\mathbf{K}_{k+1} = \mathbf{P}_k^T \mathbf{K}_k \mathbf{P}^T$$

• The Jacobi Method

A threshold Jacobi method may be formulated such that only elements larger than a certain threshold are zeroed i.e. convergence is achieved when:

$$\frac{k_{ii}^{(l+1)} - k_{ii}^{(l)}}{k_{ii}^{(l+1)}} \le 10^{-s} \quad i = 1, 2..., n \quad \text{(zero convergence)}$$
$$\frac{\left(k_{ij}^{(l+1)}\right)^{2}}{k_{ii}^{(l+1)}k_{jj}^{(l+1)}} \le 10^{-s} \quad \text{all} \quad i, j; i < j \quad \text{(coupling convergence)}$$



$$\mathbf{K}_{k+1} = \mathbf{P}_k^T \mathbf{K}_k \mathbf{P}^T$$

• The Jacobi Method

The procedure is summarized as:

Initialize the threshold for the *m*th sweep (10<sup>-2m</sup>)
 For all *i,j* with *i<j* calculate the coupling factor

$$\frac{\left(k_{ij}^{(l+1)}\right)^2}{k_{ii}^{(l+1)}k_{jj}^{(l+1)}}$$

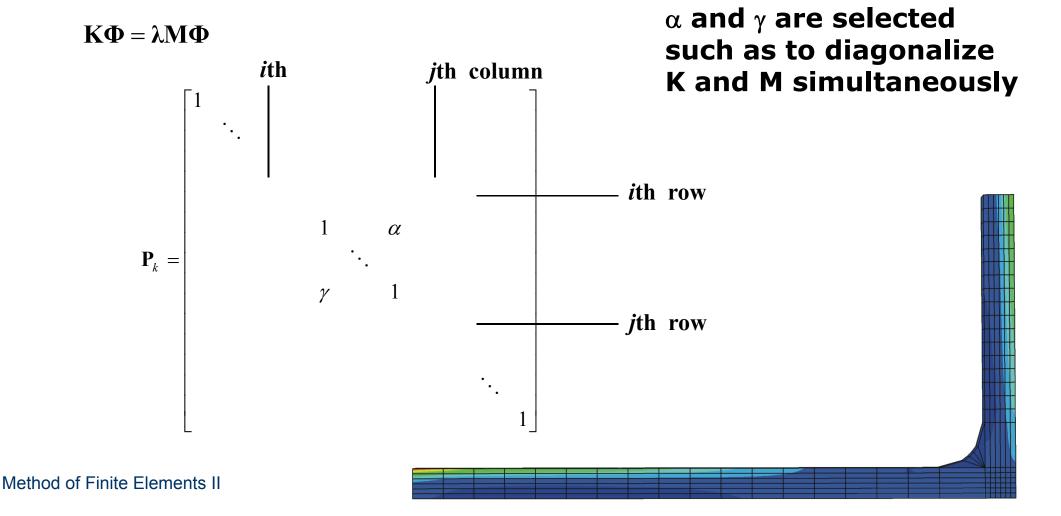
check if it is larger than the current threshold and only then apply transformation

3) Check for (zero) convergence and if fulfilled check for coupling convergence

**Convergence can be proved!** (in practice *s* = 12)



 The Generalized Jacobi Method (M ≠ I) Operates directly on K and M





The Generalized Jacobi Method (M ≠ I)

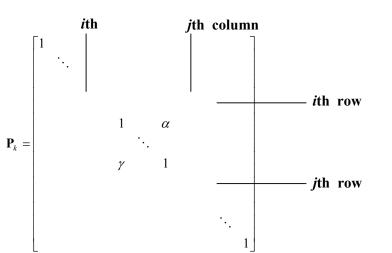
 $K\Phi = \lambda M\Phi$ 

**Performing the multiplications** 

 $\mathbf{P}_k^T \mathbf{K}_k \mathbf{P}_k \qquad \mathbf{P}_k^T \mathbf{M}_k \mathbf{P}_k$ 

and requiring that : 
$$k_{ij}^{(k+1)} = m_{ij}^{(k+1)} = 0$$

we get two equations to determine  $\alpha$  and  $\gamma$ :  $\alpha k_{ii}^{(k)} + (1 + \alpha \gamma) k_{ij}^{(k)} + \gamma k_{jj}^{(k)} = 0$   $\alpha m_{ii}^{(k)} + (1 + \alpha \gamma) m_{ij}^{(k)} + \gamma m_{jj}^{(k)} = 0$ Method of Finite Elements II



$$\alpha k_{ii}^{(k)} + (1 + \alpha \gamma) k_{ij}^{(k)} + \gamma k_{jj}^{(k)} = 0$$
  
$$\alpha m_{ii}^{(k)} + (1 + \alpha \gamma) m_{ij}^{(k)} + \gamma m_{jj}^{(k)} = 0$$

• The Generalized Jacobi Method (M ≠ I)

#### We may solve the two equations from:

$$\overline{k_{ii}}^{(k)} = k_{ii}^{(k)} m_{ij}^{(k)} - m_{ii}^{(k)} k_{ij}^{(k)}$$

$$\overline{k_{jj}}^{(k)} = k_{jj}^{(k)} m_{ij}^{(k)} - m_{jj}^{(k)} k_{ij}^{(k)}$$

$$\overline{k}^{(k)} = k_{ii}^{(k)} m_{jj}^{(k)} - k_{ii}^{(k)} m_{ii}^{(k)}$$

$$\gamma = -\frac{\overline{k_{ii}}^{(k)}}{x}; \quad \alpha = \frac{\overline{k_{jj}}^{(k)}}{x}, \qquad x = \frac{\overline{k}^{(k)}}{2} + sign(\overline{k}^{(k)}) \sqrt{\left(\frac{\overline{k}^{(k)}}{2}\right)^2 + \overline{k_{ii}}^{(k)} k_{jj}^{(k)}}$$

the iteration is performed as before but we must now check that the coupling factors are zero and that the off-diagonal elements are zero for both K and M

The Householder-QR-Inverse Iteration Solution

We consider here  $\mathbf{K}\Phi = \lambda \Phi$ 

The HQRI solution method stands for the following steps:

- 1) Householder transformations are employed to reduce the matrix K to tridiagonal form
- 2) QR iteration yields all eigenvalues
- 3) Using inverse iteration the eigenvectors of the tridiagonal matrix are calculated these vectors are then transformed into the eigenvectors of K

• The Householder-QR-Inverse Iteration Solution

The Householder transformations:

We transform K to tridiagonal form by *n*-2 transformations

$$\mathbf{K}_{k+1} = \mathbf{P}_{k}^{T} \mathbf{K}_{k} \mathbf{P}_{k}, \qquad k = 1, 2..., n-2 \qquad \mathbf{K}_{1} = \mathbf{K}$$

$$\mathbf{P}_{k} = \mathbf{I} - \boldsymbol{\theta} \mathbf{w}_{k} \mathbf{w}_{k}^{T}, \quad \boldsymbol{\theta} = \frac{2}{\mathbf{w}_{k}^{T} \mathbf{w}_{k}}$$

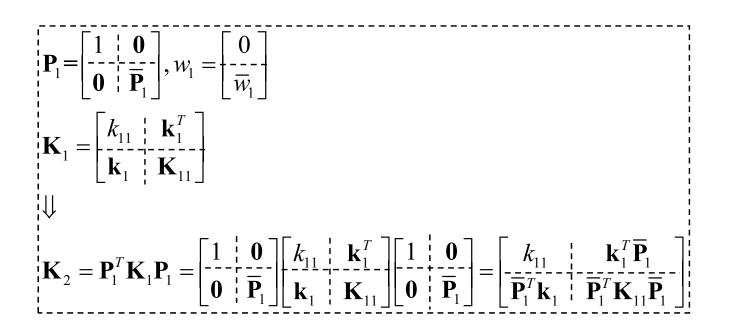
$$\uparrow$$
Reflection matrix

Method of Finite Elements II

• The Householder-QR-Inverse Iteration Solution

The Householder transformations:

We may determine w<sub>k</sub> from:

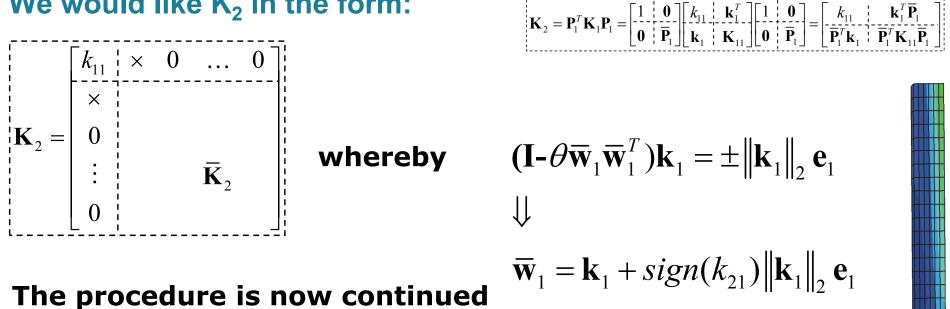




The Householder-QR-Inverse Iteration Solution

The Householder transformations:

We would like K<sub>2</sub> in the form:



 $\mathbf{P}_{1} = \begin{bmatrix} \mathbf{1} & \mathbf{0} \\ \mathbf{0} & \mathbf{\overline{P}}_{1} \end{bmatrix}, w_{1} = \begin{bmatrix} \mathbf{0} \\ -\mathbf{\overline{w}}_{1} \end{bmatrix}$ 

 $\mathbf{K}_1 = \begin{bmatrix} k_{11} & \mathbf{k}_1^T \\ \mathbf{k}_1 & \mathbf{K}_{11} \end{bmatrix}$ 

by considering  $K_2$  as  $K_1$  in the previous

- The Householder-QR-Inverse Iteration Solution
  - The **QR** iteration
  - The basic step is to decompose K into the form:

 $\mathbf{K} = \mathbf{Q}\mathbf{R}$ 

and then  $\mathbf{R}\mathbf{Q} = \mathbf{Q}^T\mathbf{K}\mathbf{Q}$ 

which is of the same form as:  $\mathbf{K}_2 = \mathbf{P}_1^T \mathbf{K}_1 \mathbf{P}_1$ 

R and Q may be found from:

$$\mathbf{P}_{n,n-1}^T \dots \mathbf{P}_{3,1}^T \mathbf{P}_{2,1}^T \mathbf{K} = \mathbf{R} \implies \mathbf{Q} = \mathbf{P}_{2,1} \mathbf{P}_{3,1} \dots \mathbf{P}_{n,n-1}$$

Method of Finite Elements II

- The Householder-QR-Inverse Iteration Solution
  - The **QR** iteration

We may now iterate:

 $\mathbf{K}_k = \mathbf{Q}_k \mathbf{R}_k$ 

 $\mathbf{K}_{k+1} = \mathbf{R}_k \mathbf{Q}_k$ 

 $\mathbf{K}_{k+1} \to \Lambda$  and  $\mathbf{Q}_1 \dots \mathbf{Q}_{k-1} \mathbf{Q}_k \to \Phi$ ,  $k \to \infty$ 

- The Householder-QR-Inverse Iteration Solution
  - The **QR** iteration

As we started out with the QR method should have been used on the tridiagonalized K matrix

If we do that we find the eigenvectors of the triangularized K matrix i.e.  $K_{n-1}$  or  $T_1$ 

Denoting the *i*th eigenvector of  $T_1$ , by  $\Psi_i$ 

we may transform back to the eigenvector of K by:

$$\mathbf{\Phi}_i = \mathbf{P}_1 \mathbf{P}_2 \dots \mathbf{P}_{n-2} \mathbf{\Psi}_i$$

#### • Non-Linear Finite Element Method

Basic principle – we sub-divide the loading in time steps, linearize the equilibrium equations (tankent stiffness matrix) and iterate for the solution (Newton Rapson/Modified NR, quasi Newton)

Cauchy stress tensor, Green-Lagrange strain tensor, Almansi strain tensor, second Piola Kirchoff stress tensor

**Total Lagrange formulation – reference to original configuration** 

**Updated Lagrange – reference to present configuration** 

**Material non-linear TL=UL** 

**Analysis of truss and beam elements** 

- Non-Linear Finite Element Method
  - **ISO-parametric elements**
  - 2D: Axisymmetric element, plane strain, plane stress
  - 3D: Solid elements, beam and axi-symmetric shell elements

Constitutive relations for non-linear materials Elasto-plastic Prandtl-Reuss van Mises (hardening/steel) Drucker-Prager (rock/soil) Thermoelastoplasticity, visco-plasticity, creep Elasto-plasticity Rate based formulations (Jaumann stress rate velocity) Contact problems



Dynamic Finite Element Method

Direct integration – subdivide tiem into steps, assume a certain variation of the motion - and integrate

**Explicit – reference to t** (no factorization) **Imlicit** – reference to  $t+\Delta t$  (factorization)

**Central differences, Houbolt, Wilson** θ, Newmark

Modal analysis (with and without damping/Rayleigh damping)

**Precision and convergence** 



• Dynamic Finite Element Method

Material non-linear dynamics – iterate within each time step based on a linearization of the stiffness matrix

**Eigenvalue problems** 

- vector iteration method
- transformation methods